

The Traffic Group



Traffic Impact Analysis

for

CROWN FARM

Gaithersburg, Maryland

Prepared for

North Gaithersburg Investment, LLC

December 8, 2004

PENGAD-Bayonne, N. J.	EXHIBIT		10/10/04
	# 57	X - 181	
		10/10/04	
PENGAD-Bayonne, N. J.	EXHIBIT		10/10/04
	57	X - 181	
		10/10/04	

Attachment "I"

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for
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Maryland*

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APPENDICES

APPENDIX A - Intersection Turning Movement Counts, Condition Diagrams and Photos, Preliminary Traffic Analysis Information

APPENDIX B - Intersection Capacity Analysis Worksheets

APPENDIX C - Background Development Information

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*GEC/clg
(F:\2004\2004-1011\wp\Report.doc)*

INTRODUCTION and SUMMARY OF FINDINGS

STUDY PURPOSE

The primary purpose of this Traffic Impact Analysis was to determine what impact the proposed development of the Crown Farm Property would have on the adjacent road network in Gaithersburg, Maryland. The Crown Farm Site is located along the west side of Sam Eig Highway, just south of its intersection with Fields Road. This property is planned to be developed with 80 townhouse units.

STUDY CRITERIA/METHODOLOGY

This Traffic Impact Analysis was conducted in accordance with a Scoping Meeting held with the City of Gaithersburg Staff. Intersection Capacity Analyses were conducted using the Critical Lane Volume (CLV) Methodology utilized by the City of Gaithersburg and the Maryland State Highway Administration. The trip generation determination for this site utilized the Institute of Transportation Engineers (ITE) Trip Generation Report (7th Edition).

SCOPE OF SERVICES

The principal scope of services undertaken as part of this study was as follows:

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- *Field investigation to collect physical information concerning the nearby road system to include condition diagrams and a photo survey.*
 - *Conduct intersection turning movement counts from 6:30 to 9:30 AM and 4 to 7 PM at each of the study area intersections.*
 - *Obtain information from the City of Gaithersburg relative to other approved developments planned in the vicinity of the subject site.*
 - *Conduct Trip Generation and Trip Distribution Analyses for the nearby developments.*
 - *Conduct Trip Generation and Trip Distribution Analyses for the proposed development of the Crown Farm.*

- *Conduct Intersection Capacity Analyses to determine existing and projected levels of service at the key intersections.*
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SUMMARY OF FINDINGS and RECOMMENDATIONS

This Traffic Impact Analysis was prepared to determine what impact the proposed development of the Crown Farm would have on the adjacent road network in Gaithersburg, Maryland. Intersection Capacity Analyses were conducted using the CLV Methodology.

The results of our analysis indicate that based on the existing traffic conditions in this area, two intersections are presently operating at unsatisfactory levels of service during the peak periods. These intersections are as follows:

- MD 119/Sam Eig Highway
- MD 119 and Muddy Branch Road

With the approved developments in this area, the study area intersections are projected to continue to operate at satisfactory levels of service with the exception of the two intersections which are currently operating at unsatisfactory levels of service. It should be pointed out that with the development of the approved developments, several roadway improvements are being made. These improvements are either in place or currently under construction. With these improvements, the intersection of MD 119 and Sam Eig Highway is projected to operate at a satisfactory level of service during both the morning and evening peak periods. The intersection of MD 119 and Muddy Branch Road however, is projected to continue to operate at Level of Service "F" conditions during both the morning and evening peak period.

The results of the analysis for the total traffic conditions indicate that all of the study area intersections are projected to continue to operate at satisfactory levels of service with the exception of the MD 119 and Muddy Branch Road intersection. This intersection is projected to operate at an unsatisfactory Level of Service "F" during both the morning and evening peak hour even when considering the improvements required of the approved background developments. It should be pointed out that the traffic projected to be generated by the Crown Farm Property has very little impact on the MD 119 and Muddy Branch Road intersection. During the morning peak period, the CLV at this intersection will not change as a result of the Crown Farm. During the evening peak hour, an increase of one (1) CLV will be experienced as a result of the development of the Crown Farm. Therefore, this project will have a negligible impact on the MD 119 and Muddy Branch Road intersection.

Based on the data contained in this report, it is our opinion that the proposed development of the Crown Farm will not have a noticeable impact at the MD 117 and Muddy Branch Road

intersection and all of the other intersections in the study area will operate at satisfactory levels of service.

The data and methodology used to undertake this study is detailed in the sections that follow.

EXISTING TRAFFIC CONDITIONS

SITE INFORMATION

The Crown Farm is located along the west side of Sam Eig Highway, south of its intersection with Fields Road as shown on Exhibit 1. Exhibits 1A and 1B show the site layout. Access to this property is planned at one location along Story Drive and one location along Bickerstaff Way.

STUDY AREA

Based on a Scoping Meeting held with the City of Gaithersburg Staff, the following intersections were identified to be included in this Traffic Impact Analysis:

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- *Sam Eig Highway and Fields Road*
 - *Sam Eig Highway and Diamondback Drive*
 - *MD 119 and Sam Eig Highway*
 - *MD 119 and Muddy Branch Road*
 - *Muddy Branch Road and Diamondback Drive*
 - *Diamondback Drive and Story Drive/Bickerstaff Way*
-
-

Sam Eig Highway is a six lane divided roadway in the vicinity of the subject site. Auxiliary turn lanes are provided at the major intersections. The intersections along Sam Eig Highway at MD 119, Diamondback Drive, and Fields Road are presently signalized. The posted speed limit along Sam Eig Highway varies between 50 and 55 MPH through the study area.

MD 119 is a four lane east/west roadway in the vicinity of the subject site. Auxiliary turn lanes are provided at the major intersections.

Muddy Branch Road is a four lane north/south roadway in the vicinity of the subject site. Auxiliary turn lanes are provided at the intersection along Muddy Branch Road.

Diamondback Drive is a four lane east/west roadway which intersects with Muddy Branch Road to the west and Sam Eig Highway to the east. The intersections of Diamondback Drive with Muddy Branch Road, Story Drive/Bickerstaff Way, and Sam Eig Highway are all presently signalized.

Story Drive and Bickerstaff Way are two lane north/south roadways in the vicinity of their intersection with Diamondback Drive.

Fields Road is a two lane east/west roadway which intersects Sam Eig Highway on the east side. The intersection of Sam Eig Highway and Fields Road is presently signalized.

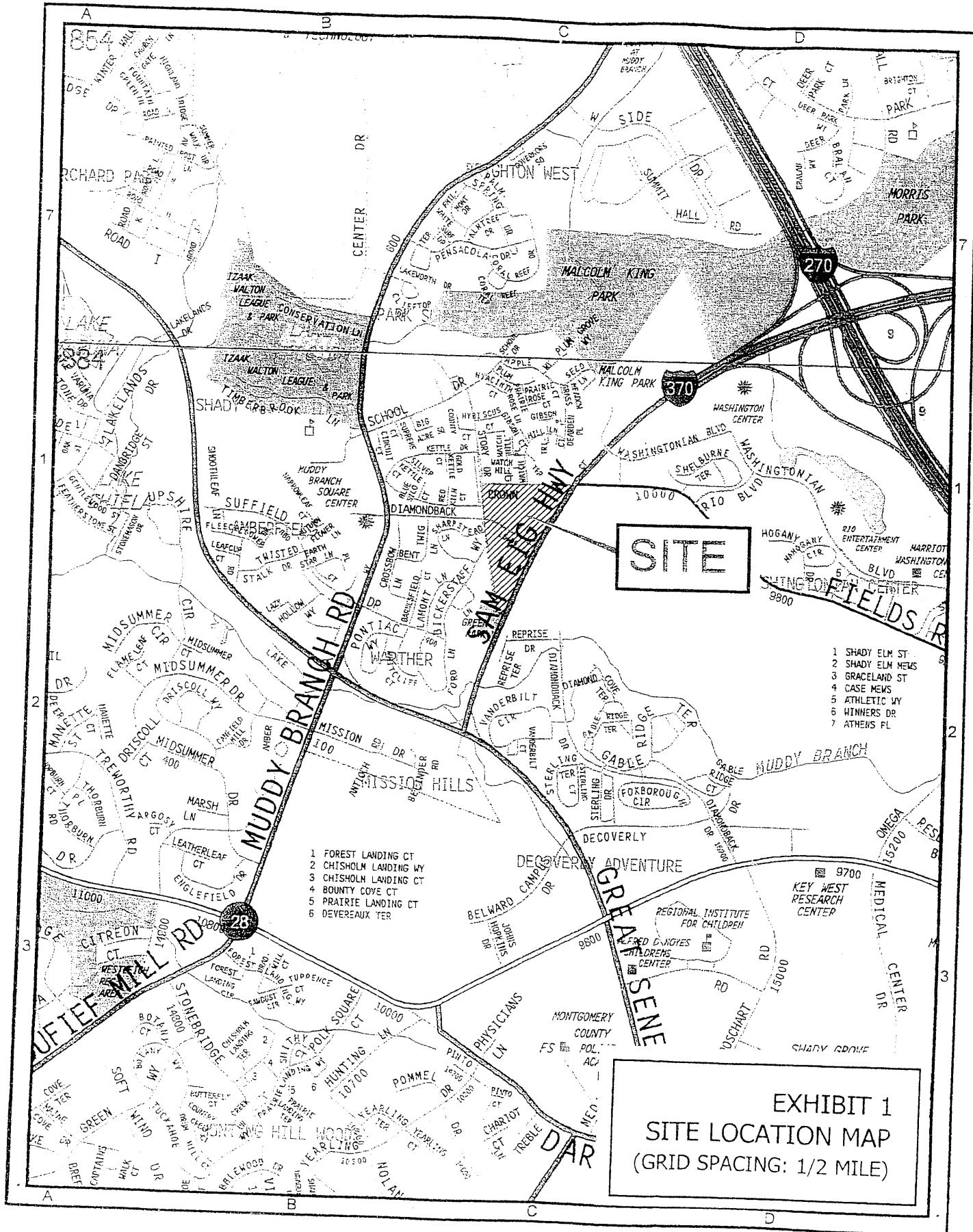
Exhibit 2 has been prepared to show the existing lane use at each of the study area intersections.

TRAFFIC VOLUMES

Intersection turning movement counts were conducted from 6:30 to 9:30 AM and 4 to 7 PM at each of the study area intersections. The total vehicles observed during these counts are shown on the summary sheets contained in Appendix A. The existing peak hour volumes are shown on Exhibit 3.

ANALYSIS OF EXISTING TRAFFIC CONDITIONS

Intersection Capacity Analyses were conducted for each of the study area intersections using the CLV Methodology and the results are shown on Exhibit 11. A review of Exhibit 11 indicates that two intersections are presently operating at a Level of Service "F" during the peak periods. The intersection of MD 119 and Sam Eig Highway is currently operating at a Level of Service "F" during the evening peak period. The intersection of MD 119 and Muddy Branch Road is currently operating at an unsatisfactory Level of Service "F" during both the morning and evening peak periods.



P_HR

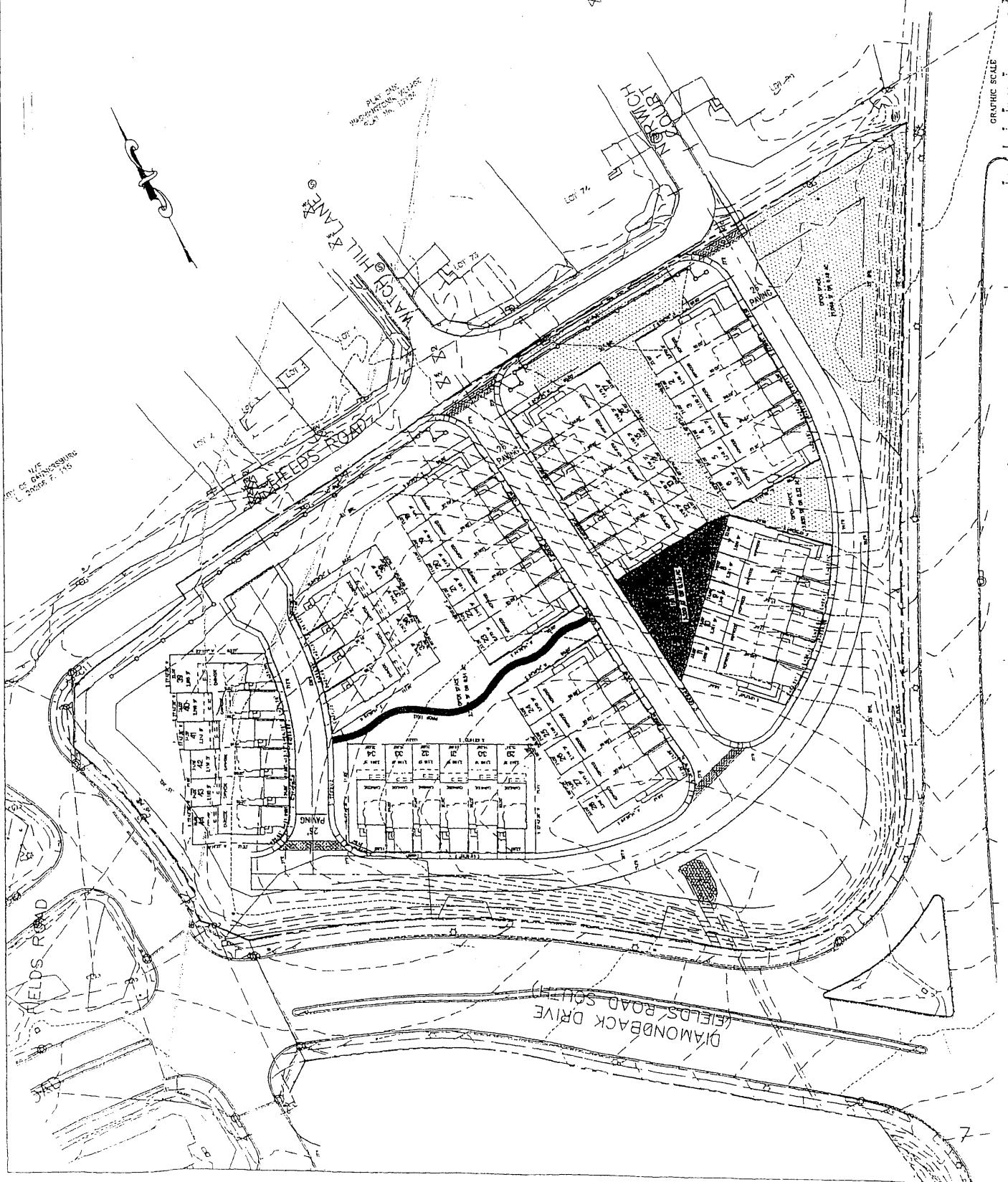
CITY OF GAITHERSBURG
SITE PLAN APPROVAL

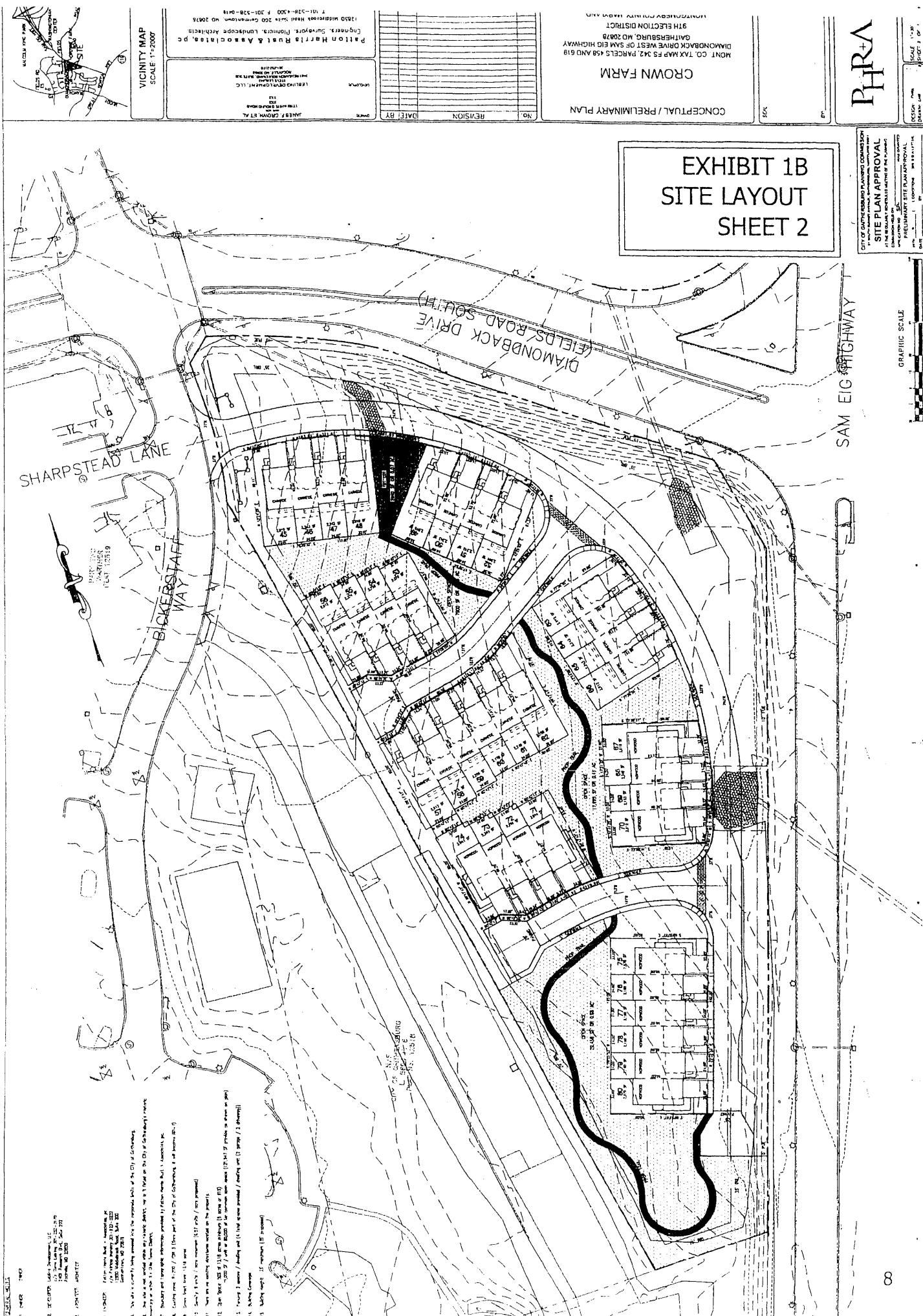
CONTRACT CO. TAX MAP FS 342 PARCELS 45 AND 619
DIAMONDBACK DRIVE WEST OF SAWYER HIGHWAY
GAITHERSBURG, MD 20878
SECTION 111, T 11 R 11 S 300 FT

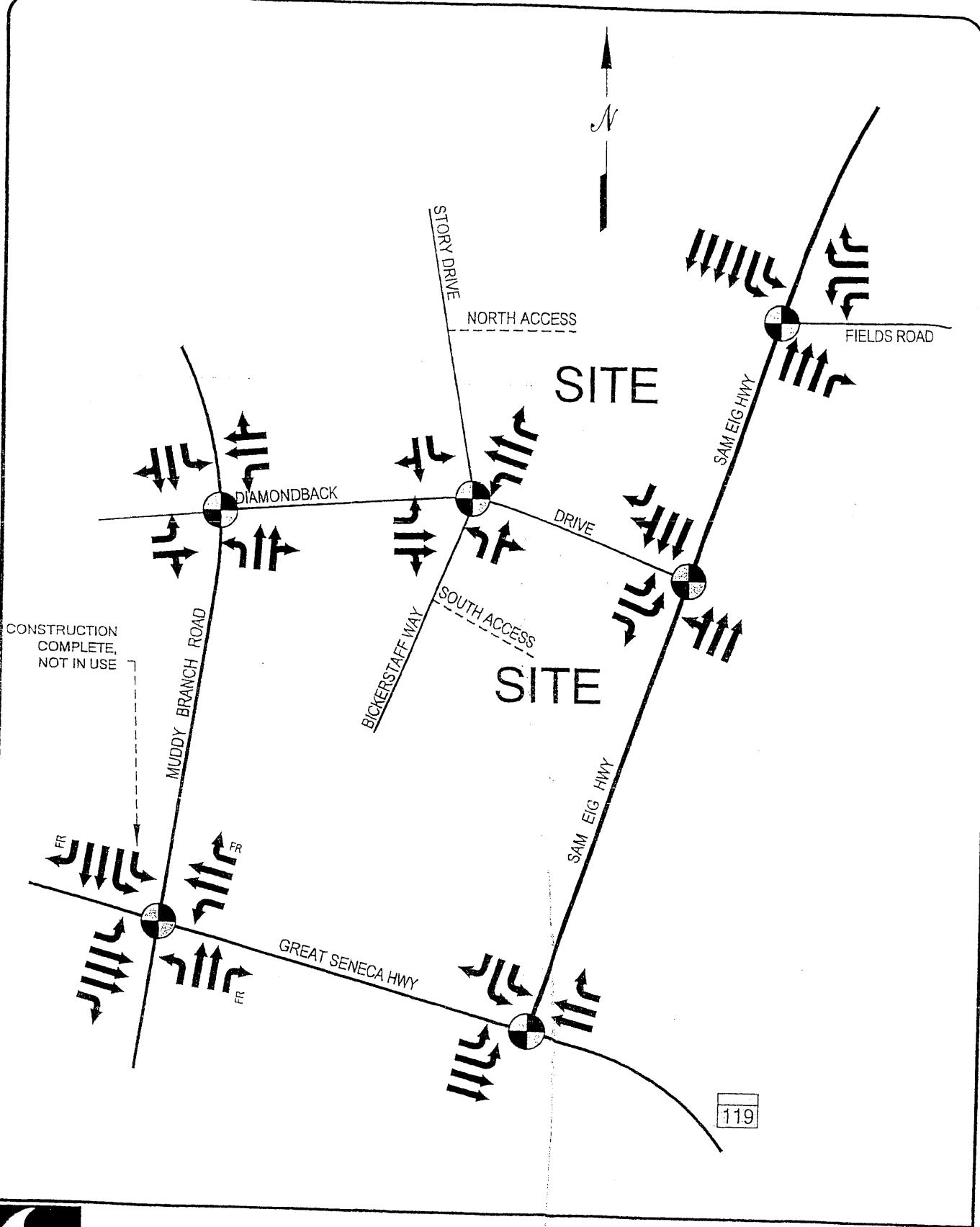
CROWN FARM

CONCEPTUAL / PRELIMINARY PLAN

EXHIBIT 1A
SITE LAYOUT
SHEET 1







NOT TO SCALE

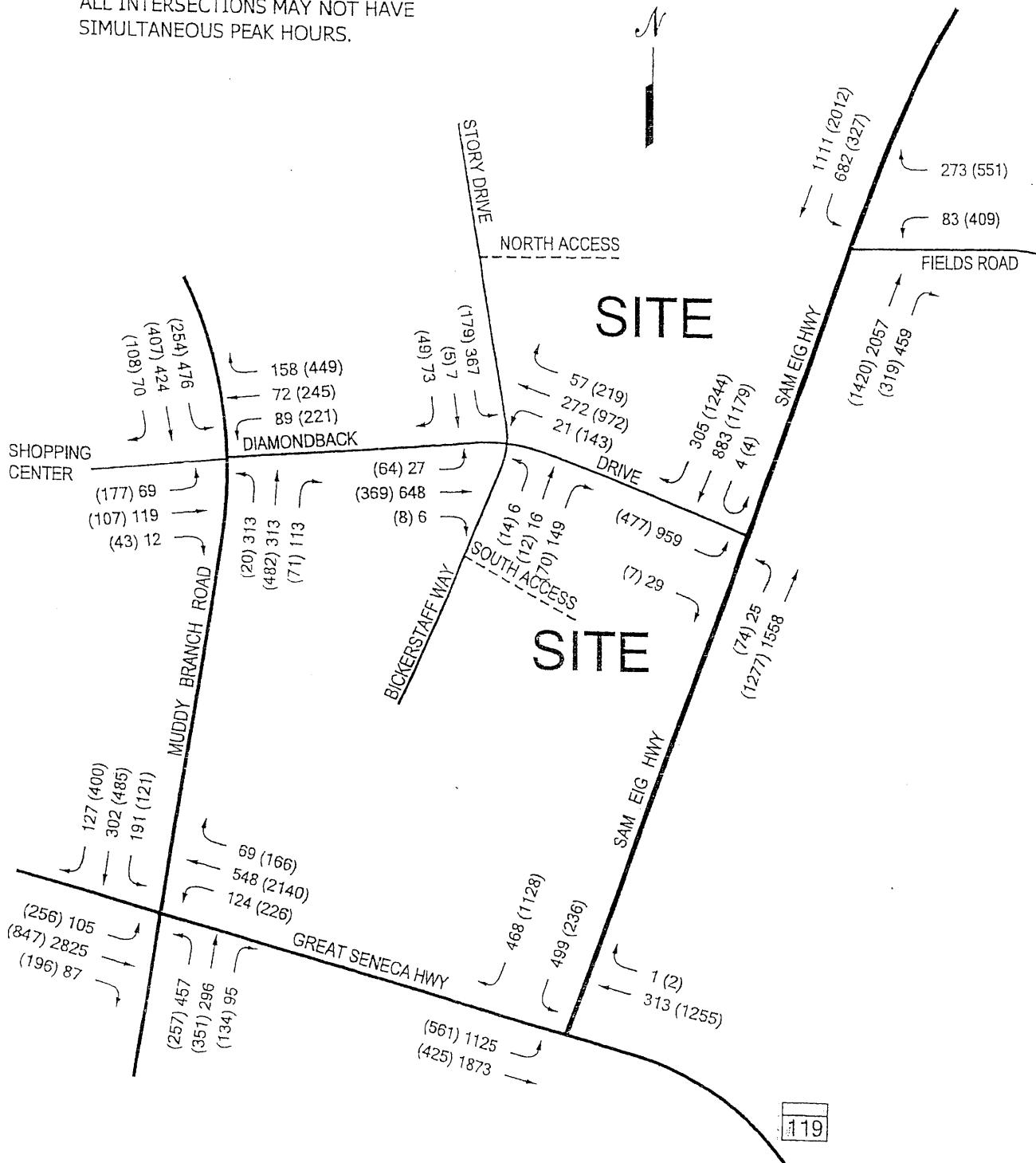


SIGNALIZED INTERSECTION

EXHIBIT 2
EXISTING LANE USE

NOTE:

ALL INTERSECTIONS MAY NOT HAVE
SIMULTANEOUS PEAK HOURS.



NOT TO SCALE

00 - MORNING PEAK HOUR
(00) - EVENING PEAK HOUR



EXHIBIT 3
EXISTING PEAK HOUR
TRAFFIC VOLUMES

BACKGROUND TRAFFIC CONDITIONS

OTHER APPROVED DEVELOPMENTS

In order to establish the background traffic conditions for this Traffic Impact Analysis, information was obtained from the City of Gaithersburg relative to other approved developments planned in the vicinity of the subject site. Based on this information, three developments were identified to be included in this analysis. They were as follows:

- Washintonian South – 350,000 square feet of office space
- Washintonian North – 850,000 square feet of office space
- Rio Waterfront – 87,815 square feet of office space – 18,080 square feet of retail space

Exhibit 4 shows the location of each of these developments.

TRIP GENERATION/DISTRIBUTION

In order to determine the impact of the other nearby developments on the adjacent road system, a Traffic Study previously prepared for the Washingtonian Center was obtained and copies of the appropriate pages are contained in Appendix C to this report. The trip generation for these developments was based on trip generation rates obtained from the ITE Trip Generation Publication.

Exhibit 5 has been prepared to show the trip generation rates and the peak hour trips projected to be generated by each of the nearby developments. These trips were then distributed and assigned to the road system based on information obtained from the Washintonian Center Traffic Impact Analysis and existing traffic patterns in this area. The peak hour trips projected to be generated by each of the nearby developments were assigned to the nearby road system as shown on the exhibits contained in Appendix C of this report. The combined peak hour trips to be generated by all of the approved developments are shown on Exhibit 6.

Combining the peak hour trips to be generated by the nearby developments with the existing peak hour volumes results in the background peak hour volumes shown on Exhibit 7.

It should be noted that several roadway improvements are either in place or under construction. These improvements include the following:

- Construction of second southbound left turn lane along Muddy Branch Road at MD 119.
- Construction of a second southbound right turn lane along Sam Eig Highway at MD 119.

ANALYSIS OF BACKGROUND TRAFFIC CONDITIONS

Intersection Capacity Analyses were conducted for the background peak hour volumes discussed above assuming the improvements that are presently under construction. The results of these analyses are shown on Exhibit 11. Copies of the capacity worksheets are contained in Appendix B of this report.

A review of Exhibit 11 indicates that based on the background peak hour volumes, all of the study area intersections are projected to operate at a satisfactory level of service during the morning and evening peak period with the exception of the MD 119 and Muddy Branch Road intersection. However, the proposed development has a negligible impact on this intersection.

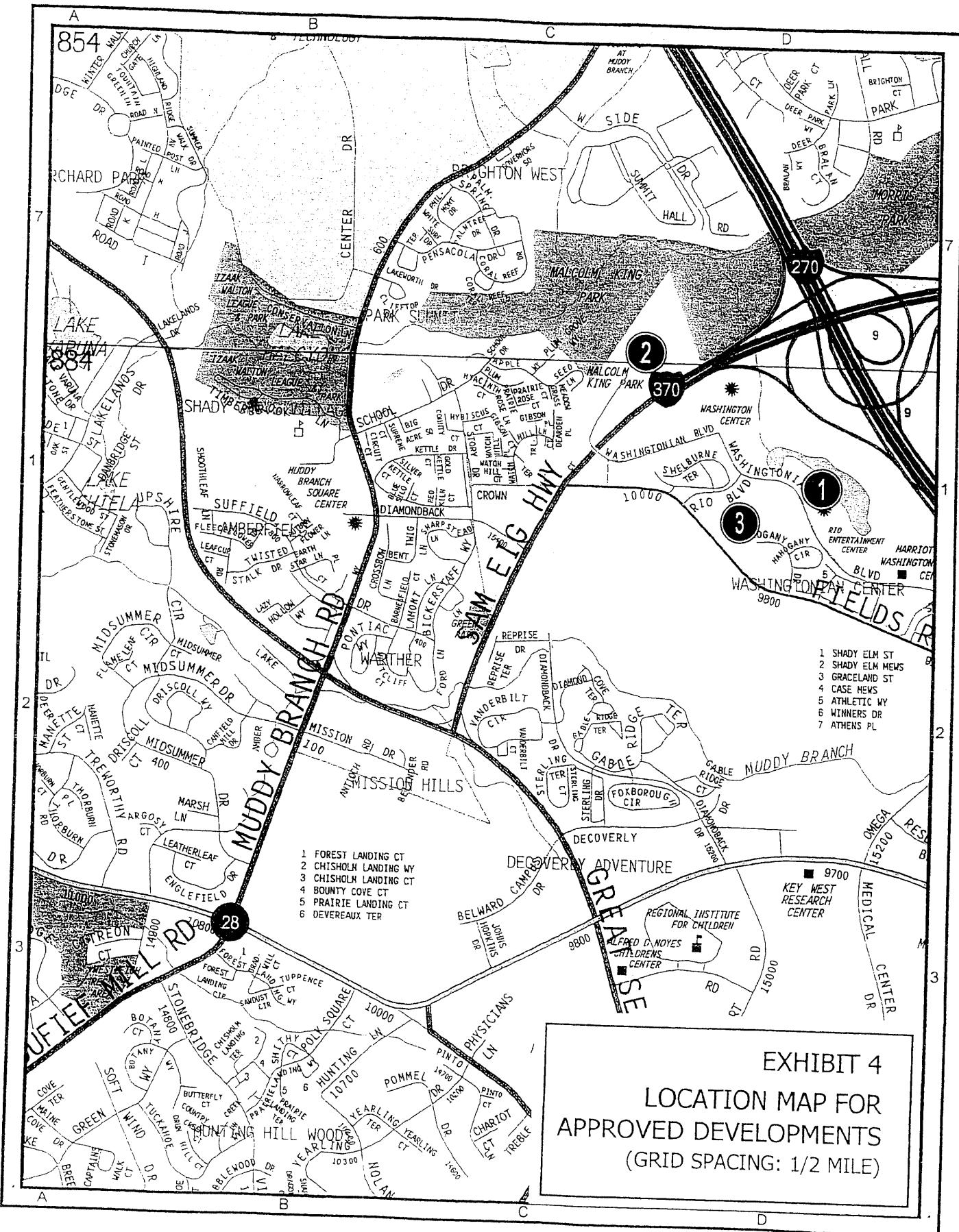


EXHIBIT 4
LOCATION MAP FOR
APPROVED DEVELOPMENTS
(GRID SPACING: 1/2 MILE)

TRIP GENERATION FOR BACKGROUND DEVELOPMENTS

TRIP RATES / FORMULAE

General Office (ksf, ITE-710)

$$\ln(\text{Morning Trips}) = 0.80 \times \ln(\text{ksf}) + 1.55 \quad \text{IN/OUT}$$

$$\text{Evening Trips} = 1.12 \times (\text{ksf}) + 78.81 \quad 88/12$$

Shopping Center (ksf, ITE-820)

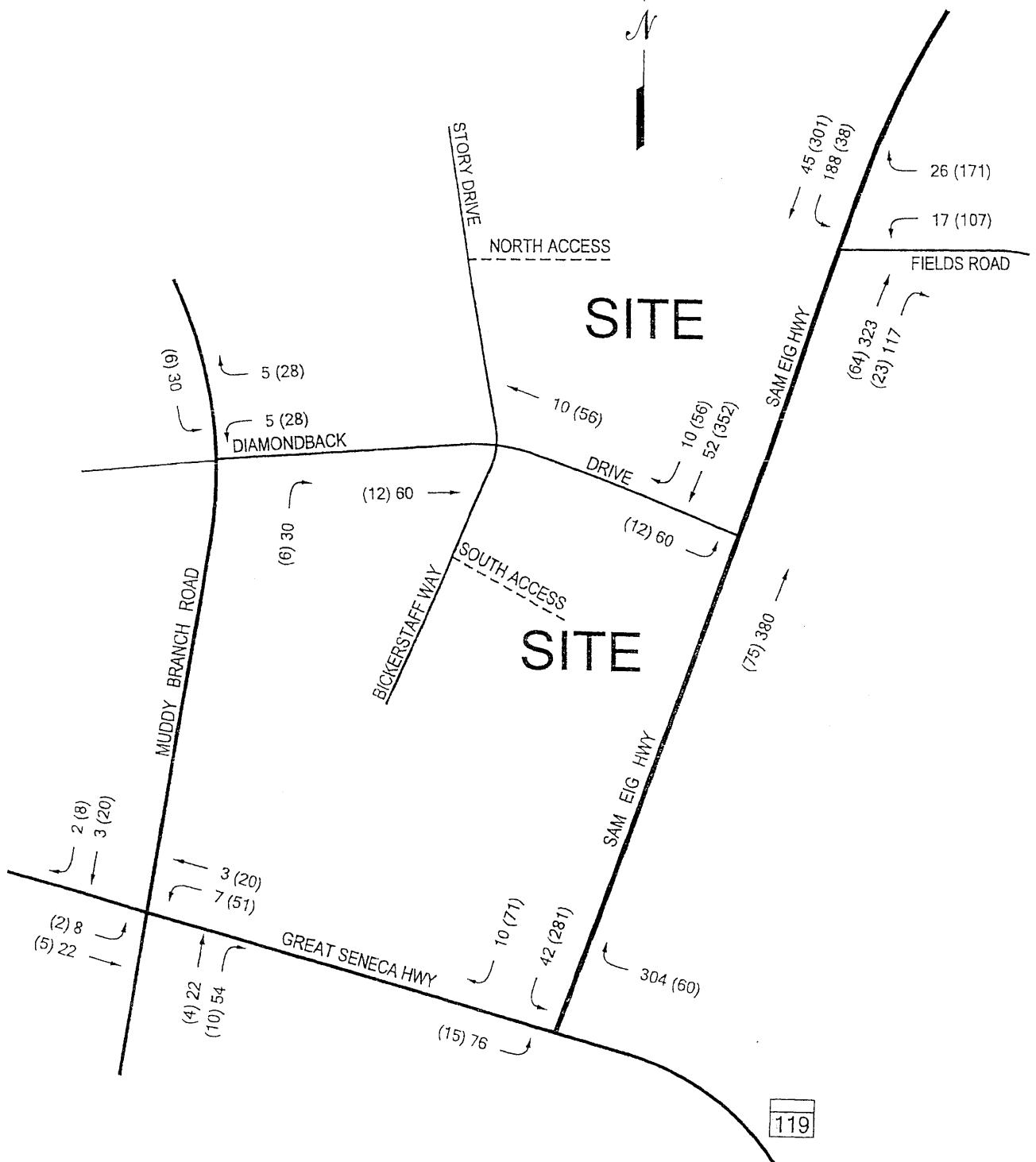
$$\text{Morning Trips} = 1.03 \times \text{ksf} \quad 61/39$$

$$\text{Evening Trips} = 3.75 \times \text{ksf} \quad 48/52$$

TRIP TOTALS

	MORNING PEAK HOUR			EVENING PEAK HOUR		
	IN	OUT	TOTAL	IN	OUT	TOTAL
1. Washingtonia South						
General Office (ksf, ITE-710)						
350,000 sq.ft.	450	61	511	80	391	471
2. Washingtonia North						
General Office (ksf, ITE-710)						
850,000 sq.ft.	914	125	1039	175	856	1031
3. Rio Waterfront						
General Office (ksf, ITE-710)						
87,815 sq.ft.	149	20	169	30	147	177
Shopping Center (ksf, ITE-820)						
18,080 sq.ft.	12	7	19	33	35	68
<u>Pass-by Trips</u>	<u>-6</u>	<u>-4</u>	<u>-10</u>	<u>-20</u>	<u>-21</u>	<u>-41</u>
<u>New Trips</u>	<u>6</u>	<u>3</u>	<u>9</u>	<u>13</u>	<u>14</u>	<u>27</u>



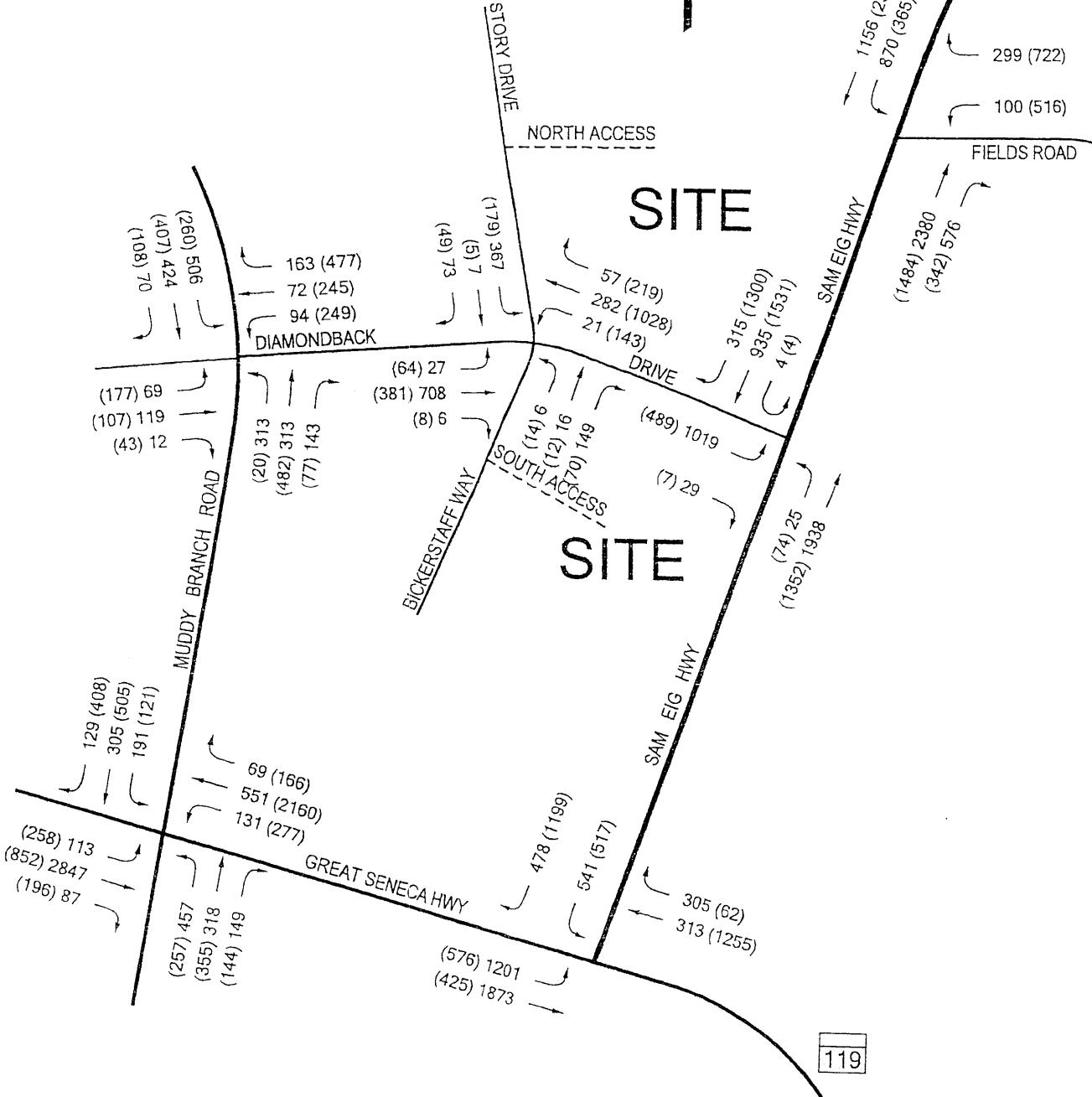


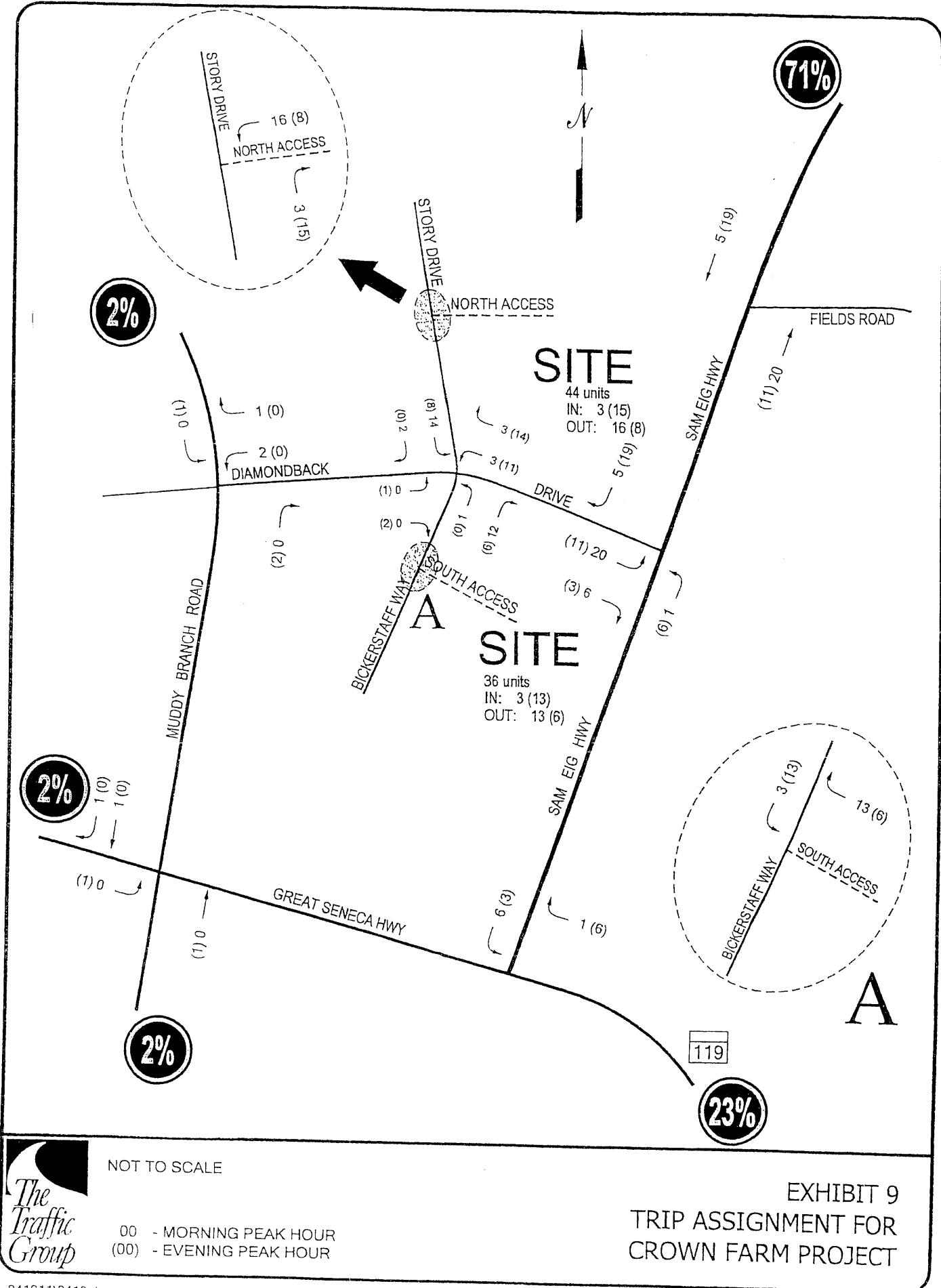
NOT TO SCALE

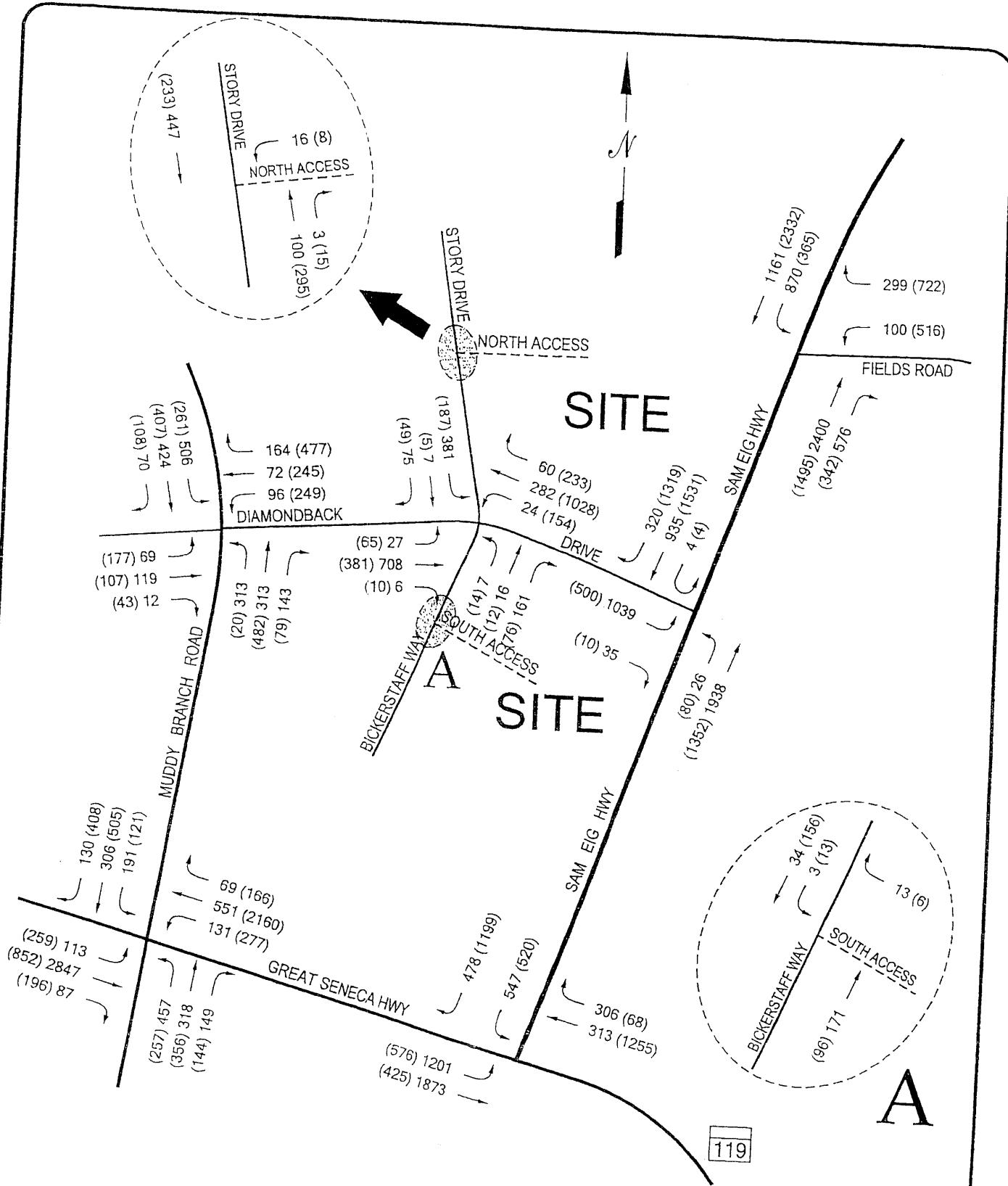


00 - MORNING PEAK HOUR
(00) - EVENING PEAK HOUR

EXHIBIT 6
COMBINED TRIPS GENERATED
BY APPROVED DEVELOPMENTS







NOT TO SCALE

00 - MORNING PEAK HOUR
(00) - EVENING PEAK HOUR

EXHIBIT 10
TOTAL PEAK HOUR
TRAFFIC VOLUMES

RESULTS, RECOMMENDATIONS and CONCLUSIONS

STUDY PURPOSE

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- MD 119 and Muddy Branch Road

With the approved developments in this area, the study area intersections are projected to continue to operate at satisfactory levels of service with the exception of the two intersections. It should be pointed out that with the development of the approved developments, several roadway improvements are being made. These improvements are either in place or currently under construction. With these improvements, the intersection of MD 119 and Sam Eig Highway is projected to operate at a satisfactory level of service during both the morning and evening peak periods. The intersection of MD 119 and Muddy Branch Road however, is projected to continue to operate at unsatisfactory Level of Service "F".

The results of the analysis for the total traffic conditions indicate that all of the study area intersections are projected to continue to operate at satisfactory levels of service with the exception of the MD 119 and Muddy Branch Road intersection. It should be pointed out that the traffic projected to be generated by the Crown Farm Property has very little impact on the MD 119 and Muddy Branch Road intersection. During the morning peak period, the CLV at this intersection will not change as a result of the Crown Farm. During the evening peak hour, an increase of one (1) CLV will be experienced as a result of the development of the Crown Farm. Therefore, this project will have almost no impact on the MD 119 and Muddy Branch Road intersection.

Therefore, based on the data contained in this report, it is our opinion that the proposed development of the Crown Farm will not have a noticeable impact at the MD 117 and Muddy Branch Road intersection and all of the other intersections in the study area will operate at satisfactory levels of service.

Please be aware the road improvements that are detailed above have not been designed. Our recommendation for these road improvements are conceptual in nature and are based upon the mathematical computations/capacity analyses that are provided in this report. It is unlikely, at this point in the process, that The Traffic Group, Inc. has undertaken sufficient field work/design to determine the impact of the recommended road improvements on either above ground or below ground utilities, drainage conditions, or right-of-way conditions that would impact the feasibility or cost of making the improvements that we have recommended. The feasibility and cost of making these improvements will be undertaken in the next phase of our studies.

RESULTS OF INTERSECTION CAPACITY ANALYSIS

MORNING PEAK HOUR TRAFFIC	EXISTING	BACKGR'D	TOTAL
1. MD 119 & Sam Eig Hwy <i>w/ planned improvement</i>	C/1294 ---	D/1317 <i>D/1317</i>	D/1320 <i>D/1320</i>
2. Sam Eig Hwy & Diamondback Dr	B/1094	C/1266	C/1278
3. Sam Eig Hwy & Fields Rd	C/1166	D/1395	D/1402
4. MD 119 & Muddy Branch Rd <i>w/ planned improvement</i>	F/1786 ---	F/1803 <i>F/1803</i>	F/1803 <i>F/1803</i>
5. Muddy Branch Rd & Diamondback Dr	A/922	A/973	A/975
6. Diamondabck Dr & Story Dr	A/900	A/931	A/960
7. Bickerstaff Way & South site access	---	---	A/187
8. Story Drive & North site access	---	---	A/463

EVENING PEAK HOUR TRAFFIC	EXISTING	BACKGR'D	TOTAL
1. MD 119 & Sam Eig Hwy <i>w/ planned improvement</i>	F/1793 ---	F/1864 <i>C/1300</i>	F/1864 <i>C/1300</i>
2. Sam Eig Hwy & Diamondback Dr	D/1322	D/1382	D/1401
3. Sam Eig Hwy & Fields Rd	A/915	B/1015	B/1019
4. MD 119 & Muddy Branch Rd <i>w/ planned improvement</i>	F/1904 ---	F/1928 <i>F/1928</i>	F/1929 <i>F/1929</i>
5. Muddy Branch Rd & Diamondback Dr	B/1092	B/1116	B/1118
6. Diamondabck Dr & Story Dr	A/840	A/870	A/885
7. Bickerstaff Way & South site access	---	---	A/175
8. Story Drive & North site access	---	---	A/318

NOTE:

1. Background Traffic is derived from combining Existing Traffic and traffic to be generated by approved developments.
2. Total Traffic is derived from combining Background Traffic and traffic to be generated by site.

EXHIBIT 11
RESULTS OF INTERSECTION
CAPACITY ANALYSIS



APPENDIX A

- *Intersection Turning Movement Counts,
Condition Diagrams and Photos*
- *Preliminary Traffic
Analysis Information*

VEHICLE TURNING MOVEMENT COUNT - SUMMARY

Intersection of: Md. 119
and: Sam Eig Hwy.
Location: Montgomery Co., Md.

Counted by: RAJ
Date: November 4, 2004
Weather: Fair, Cold
Entered by: TT
Day: Thursday



TIME	TRAFFIC FROM NORTH on: Sam Eig Hwy.					TRAFFIC FROM SOUTH on:					TRAFFIC FROM EAST on: Md. 119					TRAFFIC FROM WEST on: Md. 119					TOTAL N + S + E + W
	RIGHT	THRU	LEFT	U-TN	TOTAL	U-TN	LEFT	THRU	RIGHT	TOTAL	RIGHT	THRU	LEFT	U-TN	TOTAL	U-TN	LEFT	THRU	RIGHT	TOTAL	
AM																					
06:30-45	76	50	1	127					0	0	29	0	29	0	330	258	588	744			
45-00	76	70	0	146					0	0	35	0	35	0	290	381	671	852			
07:0-15	83	85	5	173					0	0	52	0	52	0	304	385	689	914			
15-30	69	90	1	160					0	2	33	0	35	2	309	439	750	945			
30-45	75	93	1	169					0	0	42	0	42	0	311	456	767	978			
45-00	80	114	1	195					0	0	56	0	56	0	304	470	774	1025			
08:0-15	66	114	0	180					0	0	57	0	57	0	268	483	751	988			
15-30	129	125	1	255					0	0	92	0	92	0	294	500	794	1141			
30-45	124	120	2	246					0	1	68	0	69	1	244	492	737	1052			
45-00	102	125	0	227					0	0	76	0	76	0	309	453	762	1065			
09:0-15	113	126	0	239					0	0	77	0	77	0	277	428	705	1021			
15-30	117	131	0	248					0	0	69	0	69	0	257	390	647	964			
3 Hr Totals	1110	0	1243	12	2365	0	0	0	0	0	3	686	0	0	689	3	3497	5135	0	8635	9704
1 Hr Totals																					
630-730	304	0	295	7	606	0	0	0	0	0	2	149	0	0	151	2	1233	1463	0	2698	3455
645-745	303	0	338	7	648	0	0	0	0	0	2	162	0	0	164	2	1214	1661	0	2877	3689
07-08	307	0	382	8	697	0	0	0	0	0	2	183	0	0	185	2	1228	1750	0	2980	3862
715-815	290	0	411	3	704	0	0	0	0	0	2	188	0	0	190	2	1192	1848	0	3042	3936
730-830	350	0	446	3	799	0	0	0	0	0	0	247	0	0	247	0	1177	1909	0	3086	4132
745-845	399	0	473	4	876	0	0	0	0	0	1	273	0	0	274	1	1110	1945	0	3056	4206
08-09	421	0	484	3	908	0	0	0	0	0	1	293	0	0	294	1	1115	1928	0	3044	4246
815-915	468	0	496	3	967	0	0	0	0	0	1	313	0	0	314	1	1124	1873	0	2998	4279
PEAK HOUR	456	0	502	2	960	0	0	0	0	0	1	290	0	0	291	1	1087	1763	0	2851	4102
815-915	468	0	496	3	967	0	0	0	0	0	1	313	0	0	314	1	1124	1873	0	2998	4279
PM																					
04:0-15	205	28	3	236					0	0	222	0	222	2	104	73	179	637			
15-30	212	28	2	242					0	0	273	0	273	1	143	105	249	764			
30-45	250	42	0	292					0	0	293	0	293	0	138	104	242	827			
45-00	253	40	1	294					0	0	326	0	326	0	112	95	207	827			
05:0-15	226	40	0	266					0	0	304	0	304	0	138	77	215	785			
15-30	247	39	2	288					0	0	341	0	341	0	141	112	253	882			
30-45	266	43	0	309					0	1	321	0	322	0	141	100	241	872			
45-00	278	54	3	335					0	1	337	0	338	0	160	110	270	943			
06:0-15	290	69	1	360					0	0	291	0	291	0	138	111	249	900			
15-30	294	64	2	360					0	0	306	0	306	0	122	104	226	892			
30-45	271	69	1	341					0	1	258	0	259	0	139	99	238	838			
45-00	260	55	3	318					0	0	205	0	205	0	149	73	222	745			
3 Hr Totals	3052	0	571	18	3641	0	0	0	0	3	3477	0	0	3480	3	1625	1163	0	2791	9912	
1 Hr Totals																					
04-05	920	0	138	6	1064	0	0	0	0	0	1114	0	0	1114	3	497	377	0	877	3055	
415-515	941	0	150	3	1094	0	0	0	0	0	1196	0	0	1196	1	531	381	0	913	3203	
430-530	976	0	161	3	1140	0	0	0	0	0	1264	0	0	1264	0	529	388	0	917	3321	
445-545	992	0	162	3	1157	0	0	0	0	0	1292	0	0	1293	0	532	384	0	916	3366	
05-06	1017	0	176	5	1198	0	0	0	0	0	1303	0	0	1305	0	580	399	0	979	3482	
515-615	1081	0	205	6	1292	0	0	0	0	0	1290	0	0	1292	0	580	433	0	1013	3597	
530-630	1128	0	230	6	1364	0	0	0	0	0	1255	0	0	1257	0	561	425	0	986	3607	
545-645	1133	0	256	7	1396	0	0	0	0	0	1192	0	0	1194	0	559	424	0	983	3573	
06-07	1115	0	257	7	1379	0	0	0	0	0	1060	0	0	1061	0	548	387	0	935	3375	
PEAK HOUR	530-630	1128	0	230	6	1364	0	0	0	0	0	1255	0	0	1257	0	561	425	0	986	3607

PEDESTRIAN OBSERVATIONS

Intersection of: Md. 119
and: Sam Eig Hwy.
Location: Montgomery Co., Md.

Counted by: RAJ
Date: November 4, 2004
Weather: Fair, Cold
Entered by: TT

Day: Thursday



TIME	Sam Eig Hwy.							
	NORTH LEG				SOUTH LEG			
	ADULT		CHILDREN		ADULT		CHILDREN	
PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	
AM								
06:30-45	0	0	0	0				
45-00	0	0	0	0				
07:0-15	0	0	0	0				
15-30	0	0	0	0				
30-45	0	0	0	0				
45-00	0	0	0	0				
08:0-15	0	0	0	0				
15-30	0	0	0	0				
30-45	0	0	0	0				
45-00	0	0	0	0				
09:0-15	0	0	0	0				
15-30	0	0	0	0				
TOTALS	0	0	0	0	0	0	0	0
PM								
04:0-15	0	0	0	0				
15-30	0	0	0	0				
30-45	0	0	0	0				
45-00	0	0	0	0				
05:0-15	0	0	0	0				
15-30	0	0	0	0				
30-45	0	0	0	0				
45-00	0	0	0	0				
06:0-15	0	0	0	0				
15-30	0	0	0	0				
30-45	0	0	0	0				
45-00	0	0	0	0				
TOTALS	0	0	0	0	0	0	0	0

TIME	Md. 119				Md. 119			
	EAST LEG		WEST LEG		ADULT		CHILDREN	
	ADULT	CHILDREN	ADULT	CHILDREN	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES
PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	
AM								
06:30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
07:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
08:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
09:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
TOTALS	0	0	0	0	0	0	0	0
PM								
04:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
05:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
06:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
TOTALS	0	0	0	0	0	0	0	0

VEHICLE TURNING MOVEMENT COUNT - SUMMARY

 Intersection of: Sam Eig Hwy.
 and: Diamondback Dr.
 Location: Montgomery Co., Md.

Counted by: RAJ

Date: November 3, 2004

Day: Wednesday

Weather: Fair, Cold

Entered by: TT



TIME	TRAFFIC FROM NORTH on: Sam Eig Hwy.					TRAFFIC FROM SOUTH on: Sam Eig Hwy.					TRAFFIC FROM EAST on:					TRAFFIC FROM WEST on: Diamondback Dr.					TOTAL N + S + E + W	
	RIGHT	THRU	LEFT	U-TN	TOTAL	U-TN	LEFT	THRU	RIGHT	TOTAL	RIGHT	THRU	LEFT	U-TN	TOTAL	U-TN	LEFT	THRU	RIGHT	TOTAL		
AM																						
06:30-45	29	119	0	148	0	1	330		331					0	0	215	1	216	695			
45-00	41	115	0	156	0	3	384		387					0	0	183	3	186	729			
07:0-15	51	130	1	182	0	2	395		397					0	0	225	1	226	805			
15-30	50	156	0	206	2	7	403		412					0	2	216	1	219	837			
30-45	54	156	2	212	0	2	470		472					0	0	239	0	239	923			
45-00	81	183	0	264	0	5	379		384					0	1	252	6	259	907			
08:0-15	65	234	1	300	1	4	402		407					0	0	228	9	237	944			
15-30	91	232	0	323	0	11	403		414					0	0	224	10	234	971			
30-45	68	234	3	305	0	4	374		378					0	0	254	4	258	941			
45-00	76	249	1	326	0	5	369		374					0	0	179	9	188	888			
09:0-15	65	237	0	302	0	5	320		325					0	0	138	9	147	774			
15-30	65	223	0	288	0	5	279		284					0	0	97	1	98	670			
3 Hr Totals	736	2268	0	8	3012	3	54	4508	0	4565	0	0	0	0	0	3	2450	0	54	2507	8640	
1 Hr Totals																						
630-730	171	520	0	1	692	2	13	1512	0	1527	0	0	0	0	0	2	839	0	6	847	3066	
645-745	196	557	0	3	756	2	14	1652	0	1668	0	0	0	0	0	2	863	0	5	870	3294	
07-08	236	625	0	3	864	2	16	1647	0	1665	0	0	0	0	0	3	932	0	8	943	3472	
715-815	250	729	0	3	982	3	18	1654	0	1675	0	0	0	0	0	3	935	0	16	954	3611	
730-830	291	805	0	3	1099	1	22	1654	0	1677	0	0	0	0	0	1	943	0	25	969	3745	
745-845	305	883	0	4	1192	1	24	1558	0	1583	0	0	0	0	0	1	958	0	29	988	3763	
08-09	300	949	0	5	1254	1	24	1548	0	1573	0	0	0	0	0	0	885	0	32	917	3744	
815-915	300	952	0	4	1256	0	25	1466	0	1491	0	0	0	0	0	0	795	0	32	827	3574	
830-930	274	943	0	4	1221	0	19	1342	0	1361	0	0	0	0	0	0	668	0	23	691	3273	
PEAK HOUR	745-845	305	883	0	4	1192	1	24	1558	0	1583	0	0	0	0	0	1	958	0	29	988	3763
PM																						
04:0-15	182	251	2	435	0	21	246		267					0	0	75	12	87	789			
15-30	204	258	0	462	1	11	276		288					0	0	81	3	84	834			
30-45	210	288	1	499	0	10	272		282					0	0	94	2	96	877			
45-00	248	325	0	573	0	20	273		293					0	0	94	6	100	966			
05:0-15	279	346	0	625	0	19	270		289					0	0	113	4	117	1031			
15-30	307	298	1	606	2	26	349		377					0	0	99	0	99	1082			
30-45	327	283	0	610	1	21	308		330					0	1	138	1	140	1080			
45-00	294	279	3	576	0	13	362		375					0	0	100	3	103	1054			
06:0-15	316	319	0	635	1	10	258		269					0	0	139	3	142	1046			
15-30	317	293	1	611	0	11	333		344					0	0	116	4	120	1075			
30-45	246	358	0	604	0	17	277		294					0	1	117	3	121	1019			
45-00	259	319	1	579	0	10	275		285					0	0	142	3	145	1009			
3 Hr Totals	3189	3617	0	9	6815	5	189	3499	0	3693	0	0	0	0	0	2	1308	0	44	1354	11862	
1 Hr Totals																						
04-05	844	1122	0	3	1969	1	62	1067	0	1130	0	0	0	0	0	0	344	0	23	367	3466	
415-515	941	1217	0	1	2159	1	60	1091	0	1152	0	0	0	0	0	0	382	0	15	397	3708	
430-530	1044	1257	0	2	2303	2	75	1164	0	1241	0	0	0	0	0	0	400	0	12	412	3956	
445-545	1161	1252	0	1	2414	3	86	1200	0	1289	0	0	0	0	0	1	444	0	11	456	4159	
05-06	1207	1206	0	4	2417	3	79	1289	0	1371	0	0	0	0	0	1	450	0	8	459	4247	
515-615	1244	1179	0	4	2427	4	70	1277	0	1351	0	0	0	0	0	1	476	0	7	484	4262	
530-630	1254	1174	0	4	2432	2	55	1261	0	1318	0	0	0	0	0	1	493	0	11	505	4255	
545-645	1173	1249	0	4	2426	1	51	1230	0	1282	0	0	0	0	0	1	472	0	13	486	4194	
06-07	1138	1289	0	2	2429	1	48	1143	0	1192	0	0	0	0	0	1	514	0	13	528	4149	
PEAK HOUR	515-615	1244	1179	0	4	2427	4	70	1277	0	1351	0	0	0	0	0	1	476	0	7	484	4262

PEDESTRIAN OBSERVATIONS

Intersection of: Sam Eig Hwy.
and: Diamondback Dr.
Location: Montgomery Co., Md.

Counted by: RAJ
Date: November 3, 2004
Weather: Fair, Cold
Entered by: TT

Day: Wednesday



TIME	Sam Eig Hwy.				Sam Eig Hwy.			
	NORTH LEG				SOUTH LEG			
	ADULT		CHILDREN		ADULT		CHILDREN	
	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES
AM								
06:30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
07:0-15	2	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	2	0	0	0	0	0	0	0
45-00	1	0	0	0	0	0	0	0
08:0-15	0	0	0	0	0	0	0	0
15-30	1	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
09:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
TOTALS	6	0	0	0	0	0	0	0
PM								
04:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	1	0	0	0	0	0	0	0
05:0-15	0	0	0	0	0	0	0	0
15-30	2	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
06:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
TOTALS	3	0	0	0	0	0	0	0

TIME	EAST LEG				Diamondback Dr.			
	ADULT		CHILDREN		ADULT		CHILDREN	
	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES
AM								
06:30-45					0	0	0	0
45-00					0	0	0	0
07:0-15					0	0	0	0
15-30					0	0	0	0
30-45					0	0	0	0
45-00					0	0	0	0
08:0-15					0	0	0	0
15-30					0	0	0	0
30-45					0	0	0	0
45-00					0	0	0	0
09:0-15					0	0	0	0
15-30					0	0	0	0
TOTALS	0	0	0	0	0	0	0	0
PM								
04:0-15					0	0	0	0
15-30					0	0	0	0
30-45					0	0	0	0
45-00					0	0	0	0
05:0-15					0	0	0	0
15-30					0	0	0	0
30-45					0	0	0	0
45-00					0	0	0	0
06:0-15					0	0	0	0
15-30					0	0	0	0
30-45					0	0	0	0
45-00					0	0	0	0
TOTALS	0	0	0	0	0	0	0	0

VEHICLE TURNING MOVEMENT COUNT - SUMMARY

Intersection of: Sam Eig Hwy.
and: Fields Rd.
Location: Montgomery Co., Md.

Counted by: RAJ

Date: November 3, 2004

Day: Tuesday

Weather: Fair, Cold

Entered by: TT



TIME	TRAFFIC FROM NORTH on: Sam Eig Hwy.					TRAFFIC FROM SOUTH on: Sam Eig Hwy.					TRAFFIC FROM EAST on: Fields Rd.					TRAFFIC FROM WEST on:					TOTAL N + S + E + W	
	RIGHT	THRU	LEFT	U-TN	TOTAL	U-TN	LEFT	THRU	RIGHT	TOTAL	RIGHT	THRU	LEFT	U-TN	TOTAL	U-TN	LEFT	THRU	RIGHT	TOTAL		
AM																						
06:30-45	131	75	0	206	0	491	54	545	42	17	0	59									0	810
45-00	136	79	0	215	0	507	60	567	45	20	1	66									0	848
07:0-15	162	70	0	232	0	560	60	620	55	19	0	74									0	926
15-30	190	148	1	339	0	533	86	619	50	16	1	67									0	1025
30-45	190	124	4	318	0	609	100	709	53	20	0	73									0	1100
45-00	247	202	2	451	0	520	111	631	84	17	1	102									0	1184
08:0-15	276	159	1	436	0	507	123	630	55	23	0	78									0	1144
15-30	299	161	2	462	0	509	118	627	79	24	4	107									0	1196
30-45	289	154	1	444	0	521	107	628	55	13	1	69									0	1141
45-00	301	210	1	512	0	450	98	548	73	24	0	97									0	1157
09:0-15	281	121	0	402	0	375	83	458	61	21	1	83									0	943
15-30	256	115	0	371	0	319	57	376	59	32	2	93									0	840
3 Hr Totals	0	2758	1618	12	4388	0	0	5901	1057	6958	711	0	246	11	968	0	0	0	0	0	10531	
1 Hr Totals																						
630-730	0	619	372	1	992	0	0	2091	260	2351	192	0	72	2	266	0	0	0	0	0	3609	
645-745	0	678	421	5	1104	0	0	2209	306	2515	203	0	75	2	280	0	0	0	0	0	3899	
07-08	0	789	544	7	1340	0	0	2222	357	2579	242	0	72	2	316	0	0	0	0	0	4235	
715-815	0	903	633	8	1544	0	0	2169	420	2589	242	0	76	2	320	0	0	0	0	0	4453	
730-830	0	1012	646	9	1667	0	0	2145	452	2597	271	0	84	5	360	0	0	0	0	0	4624	
745-845	0	1111	676	6	1793	0	0	2057	459	2516	273	0	77	6	356	0	0	0	0	0	4665	
08-09	0	1165	684	5	1854	0	0	1987	446	2433	262	0	84	5	351	0	0	0	0	0	4638	
815-915	0	1170	646	4	1820	0	0	1855	406	2261	268	0	82	6	356	0	0	0	0	0	4437	
830-930	0	1127	600	2	1729	0	0	1665	345	2010	248	0	90	4	342	0	0	0	0	0	4081	
PEAK HOUR	745-845	0	1111	676	6	1793	0	0	2057	459	2516	273	0	77	6	356	0	0	0	0	0	4665
PM																						
04:0-15	390	54	1	445	1	274	47	322	103	43	1	147								0	914	
15-30	402	58	0	460	1	313	44	358	103	60	0	163								0	981	
30-45	421	51	0	472	0	321	45	366	126	77	0	203								0	1041	
45-00	503	71	0	574	0	305	62	367	121	70	1	192								0	1133	
05:0-15	514	99	0	613	0	306	77	383	154	111	1	266								0	1262	
15-30	509	69	1	579	0	363	85	448	144	96	1	241								0	1268	
30-45	510	85	1	596	0	371	75	446	140	100	1	241								0	1283	
45-00	479	72	0	551	0	380	82	462	113	94	5	212								0	1225	
06:0-15	542	101	0	643	0	337	60	397	116	93	0	209								0	1249	
15-30	517	71	0	588	0	363	86	449	90	93	7	190								0	1227	
30-45	526	68	0	594	0	313	81	394	109	78	0	187								0	1175	
45-00	521	70	1	592	0	347	70	417	80	57	2	139								0	1148	
3 Hr Totals	0	5834	869	4	6707	2	0	3993	814	4809	1399	0	972	19	2390	0	0	0	0	0	13906	
1 Hr Totals																						
04-05	0	1716	234	1	1951	2	0	1213	198	1413	453	0	250	2	705	0	0	0	0	0	4069	
415-515	0	1840	279	0	2119	1	0	1245	228	1474	504	0	318	2	824	0	0	0	0	0	4417	
430-530	0	1947	290	1	2238	0	0	1295	269	1564	545	0	354	3	902	0	0	0	0	0	4704	
445-545	0	2036	324	2	2362	0	0	1345	299	1644	559	0	377	4	940	0	0	0	0	0	4946	
05-06	0	2012	325	2	2339	0	0	1420	319	1739	551	0	401	8	960	0	0	0	0	0	5038	
515-615	0	2040	327	2	2369	0	0	1451	302	1753	513	0	383	7	903	0	0	0	0	0	5025	
530-630	0	2048	329	1	2378	0	0	1451	303	1754	459	0	380	13	852	0	0	0	0	0	4984	
545-645	0	2064	312	0	2376	0	0	1393	309	1702	428	0	358	12	798	0	0	0	0	0	4876	
06-07	0	2106	310	1	2417	0	0	1360	297	1657	395	0	321	9	725	0	0	0	0	0	4799	
PEAK HOUR	05-06	0	2012	325	2	2339	0	0	1420	319	1739	551	0	401	8	960	0	0	0	0	0	5038

PEDESTRIAN OBSERVATIONS

Intersection of: Sam Eig Hwy.
and: Fields Rd.
Location: Montgomery Co., Md.

Counted by: RAJ
Date: November 3, 2004
Weather: Fair, Cold
Entered by: TT



TIME	Sam Eig Hwy.				Sam Eig Hwy.			
	NORTH LEG		SOUTH LEG					
	ADULT		CHILDREN		ADULT		CHILDREN	
	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES
AM								
06:30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
07:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
08:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
09:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
TOTALS	0	0	0	0	0	0	0	0
PM								
04:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
05:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
06:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
TOTALS	0	0	0	0	0	0	0	0

TIME	Fields Rd.				WEST LEG			
	EAST LEG		WEST LEG		ADULT		CHILDREN	
	ADULT	CHILDREN	ADULT	CHILDREN	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES
	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES				
AM								
06:30-45	0	0	0	0				
45-00	0	0	0	0				
07:0-15	2	0	0	0				
15-30	2	0	0	0				
30-45	0	0	0	0				
45-00	1	0	0	0				
08:0-15	0	0	0	0				
15-30	1	0	0	0				
30-45	0	0	0	0				
45-00	0	0	0	0				
09:0-15	0	0	0	0				
15-30	0	0	0	0				
TOTALS	6	0	0	0	0	0	0	0
PM								
04:0-15	0	0	0	0				
15-30	0	0	0	0				
30-45	0	0	0	0				
45-00	1	0	0	0				
05:0-15	0	0	0	0				
15-30	2	0	0	0				
30-45	0	0	0	0				
45-00	0	0	0	0				
06:0-15	0	0	0	0				
15-30	0	0	0	0				
30-45	0	0	0	0				
45-00	0	0	0	0				
TOTALS	3	0	0	0	0	0	0	0

VEHICLE TURNING MOVEMENT COUNT - SUMMARY

Intersection of: Md. 119
and: Muddy Branch Rd.
Location: Montgomery Co., Md.

Counted by: AN, LL

Date: November 10, 2004

Day: Wednesday

Weather: Fair, Cold

Entered by: TT



TIME	TRAFFIC FROM NORTH on: Muddy Branch Rd.					TRAFFIC FROM SOUTH on: Muddy Branch Rd.					TRAFFIC FROM EAST on: Md. 119					TRAFFIC FROM WEST on: Md. 119					TOTAL N + S + E + W	
	RIGHT	THRU	LEFT	U-TN	TOTAL	U-TN	LEFT	THRU	RIGHT	TOTAL	RIGHT	THRU	LEFT	U-TN	TOTAL	U-TN	LEFT	THRU	RIGHT	TOTAL		
AM																						
06:30-45	7	33	22	0	62	0	60	31	9	100	9	64	20	0	93	0	18	554	6	578	833	
45-00	29	50	53	0	132	0	75	34	10	119	11	87	14	0	112	0	24	685	13	722	1085	
07:0-15	32	51	42	0	125	0	70	50	11	131	11	93	18	0	122	0	21	685	18	724	1102	
15-30	47	51	49	0	147	0	86	56	9	151	15	94	16	0	125	0	18	675	14	707	1130	
30-45	25	44	39	0	108	0	95	70	21	186	21	83	19	0	123	0	30	776	15	821	1238	
45-00	28	72	45	0	145	0	119	77	22	218	12	118	31	0	161	1	32	740	23	796	1320	
08:0-15	18	68	51	0	137	0	115	76	17	208	12	131	31	0	174	0	26	675	13	714	1233	
15-30	38	84	53	0	175	0	119	72	30	221	18	146	32	0	196	0	23	718	23	764	1356	
30-45	43	78	42	0	163	0	104	71	26	201	27	153	30	0	210	0	23	692	28	743	1317	
45-00	33	71	50	0	154	0	104	85	30	219	27	161	31	0	219	0	30	605	34	669	1261	
09:0-15	29	51	38	0	118	0	93	87	29	209	24	150	29	0	203	1	19	568	21	609	1139	
15-30	27	52	32	0	111	0	93	73	37	203	21	146	27	0	194	0	34	565	23	622	1130	
3 Hr Totals	356	705	516	0	1577	0	1133	782	251	2166	208	1426	298	0	1932	2	298	7938	231	8469	11875	
1 Hr Totals																						
630-730	115	185	166	0	466	0	291	171	39	501	46	338	68	0	452	0	81	2599	51	2731	4150	
645-745	133	196	183	0	512	0	326	210	51	587	58	357	67	0	482	0	93	2821	60	2974	4555	
07-08	132	218	175	0	525	0	370	253	63	686	59	388	84	0	531	1	101	2876	70	3048	4790	
715-815	118	235	184	0	537	0	415	279	69	763	60	426	97	0	583	1	106	2866	65	3038	4921	
730-830	109	268	188	0	565	0	448	295	90	833	63	478	113	0	654	1	111	2909	74	3095	5147	
745-845	127	302	191	0	620	0	457	296	95	848	69	548	124	0	741	1	104	2825	87	3017	5226	
08-09	132	301	196	0	629	0	442	304	103	849	84	591	124	0	799	0	102	2690	98	2890	5167	
815-915	143	284	183	0	610	0	420	315	115	850	96	610	122	0	828	1	95	2583	106	2785	5073	
PEAK HOUR	132	252	162	0	546	0	394	316	122	832	99	610	117	0	826	1	106	2430	106	2643	4847	
745 345	127	302	191	0	620	0	457	296	95	848	69	548	124	0	741	1	104	2825	87	3017	5226	
PM																						
04:0-15	47	79	21	0	147	0	56	60	14	130	52	347	46	0	445	1	41	184	30	256	978	
15-30	57	96	28	0	181	0	78	89	35	202	52	427	49	1	529	0	28	197	29	254	1166	
30-45	70	108	23	0	201	0	47	62	24	133	39	498	64	0	601	0	43	190	34	267	1202	
45-00	68	87	29	0	184	0	64	88	25	177	38	444	66	0	548	0	45	193	37	275	1184	
05:0-15	94	120	26	0	240	0	52	95	35	182	56	503	67	1	627	1	59	210	36	306	1355	
15-30	80	128	26	0	234	1	62	102	38	203	41	526	62	1	630	0	67	197	43	307	1374	
30-45	79	126	34	0	239	0	59	83	34	176	39	542	56	0	637	0	61	217	41	319	1371	
45-00	128	117	29	0	274	0	63	85	28	176	45	543	54	1	643	0	82	209	59	350	1443	
06:0-15	94	84	28	0	206	0	70	96	35	201	38	543	53	1	635	0	54	211	34	299	1341	
15-30	99	158	30	0	287	0	65	87	37	189	44	512	59	2	617	0	59	210	62	331	1424	
30-45	112	97	28	0	237	0	63	97	31	191	47	467	64	1	579	0	65	187	46	298	1305	
45-00	78	100	24	1	203	0	65	67	36	168	45	433	73	1	552	0	55	184	27	266	1189	
3 Hr Totals	1006	1300	326	1	2633	1	744	1011	372	2128	536	5785	713	9	7043	2	659	2389	478	3528	15332	
1 Hr Totals																						
04-05	242	370	101	0	713	0	245	299	98	642	181	1716	225	1	2123	1	157	764	130	1052	4530	
415-515	289	411	106	0	806	0	241	334	119	694	185	1872	246	2	2305	1	175	790	136	1102	4907	
430-530	312	443	104	0	859	1	225	347	122	695	174	1971	259	2	2406	1	214	790	150	1155	5115	
445-545	321	461	115	0	897	1	237	368	132	738	174	2015	251	2	2442	1	232	817	157	1207	5284	
05-06	381	491	115	0	987	1	235	365	135	737	181	2114	239	3	2537	1	269	833	179	1282	5543	
515-615	381	455	117	0	953	1	254	366	135	756	163	2154	225	3	2545	0	264	834	177	1275	5529	
530-630	400	485	121	0	1006	0	257	351	134	742	166	2140	222	4	2532	0	256	847	196	1299	5579	
545-645	433	456	115	0	1004	0	261	365	131	757	174	2065	230	5	2474	0	260	817	201	1278	5513	
06-07	383	439	110	1	933	0	263	347	139	749	174	1955	249	5	2383	0	233	792	169	1194	5259	
PEAK HOUR	530-630	400	485	121	0	1006	0	257	351	134	742	166	2140	222	4	2532	0	256	847	196	1299	5579

PEDESTRIAN OBSERVATIONS

Intersection of: Md. 119
and: Muddy Branch Rd.
Location: Montgomery Co., Md.

Counted by: AN, LL

Day: Wednesday

Date: November 10, 2004
Weather: Fair, Cold
Entered by: TT



TIME	Muddy Branch Rd.				Muddy Branch Rd.			
	NORTH LEG		SOUTH LEG					
	ADULT	CHILDREN	ADULT	CHILDREN	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES
PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	
AM								
06:30-45	3	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
07:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
08:0-15	0	2	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	1	0	0	0
09:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
TOTALS	3	2	0	0	1	0	0	0
PM								
04:0-15	0	0	0	1	3	0	0	0
15-30	1	0	0	0	0	0	0	0
30-45	1	1	0	0	0	1	0	0
45-00	0	0	0	0	0	0	0	0
05:0-15	0	0	0	0	0	0	0	0
15-30	0	1	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
06:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
TOTALS	2	2	0	1	3	1	0	0

TIME	Md. 119				Md. 119			
	EAST LEG		WEST LEG					
	ADULT	CHILDREN	ADULT	CHILDREN	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES
PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	
AM								
06:30-45	0	0	0	0	4	0	0	0
45-00	0	0	0	0	2	0	0	0
07:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	1	0
45-00	0	0	0	0	2	0	0	0
08:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	1	0	0	0
30-45	0	0	0	0	1	0	0	0
45-00	0	0	0	0	0	0	0	0
09:0-15	0	1	0	0	0	0	0	0
15-30	0	0	0	0	1	1	0	0
TOTALS	0	1	0	0	11	1	1	0
PM								
04:0-15	1	0	0	0	0	0	0	0
15-30	0	0	0	0	1	1	0	0
30-45	0	0	0	0	0	1	0	0
45-00	0	0	0	0	0	1	0	0
05:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	2	0	2	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	1	0	0	0
06:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	1	1	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
TOTALS	1	0	0	0	5	3	1	0

VEHICLE TURNING MOVEMENT COUNT - SUMMARY

Intersection of: Muddy Branch Rd.
and: Diamondback Dr. / Shop. Ctr. Access
Location: Montgomery Co., Md.

Counted by: FC, TP

Date: November 10, 2004

Weather: Fair, Cold

Day: Wednesday

Entered by: TT



TIME	TRAFFIC FROM NORTH on: Muddy Branch Rd.					TRAFFIC FROM SOUTH on: Muddy Branch Rd.					TRAFFIC FROM EAST on: Diamondback Dr.					TRAFFIC FROM WEST on: Shop. Ctr. Access					TOTAL N + S + E + W
	RIGHT	THRU	LEFT	U-TN	TOTAL	U-TN	LEFT	THRU	RIGHT	TOTAL	RIGHT	THRU	LEFT	U-TN	TOTAL	U-TN	LEFT	THRU	RIGHT	TOTAL	
	AM																				
06:30-45	2	51	70	0	123	0	29	29	13	71	14	8	8	0	30	0	6	19	0	25	249
45-00	3	87	62	0	152	0	47	47	27	121	19	9	6	0	34	0	2	14	0	16	323
07:0-15	14	78	78	0	170	0	33	33	20	86	19	15	16	0	50	0	9	28	1	38	344
15-30	10	89	116	0	215	0	54	54	31	139	20	14	9	0	43	1	5	33	1	40	437
30-45	10	78	125	2	215	0	64	64	32	160	25	13	19	0	57	0	12	33	1	46	478
45-00	17	99	138	0	254	0	82	82	25	189	34	12	12	0	58	0	7	29	2	38	539
08:0-15	20	94	131	1	246	0	61	61	31	153	43	22	17	0	82	0	15	30	1	46	527
15-30	15	117	141	0	273	0	80	80	33	193	35	19	17	0	71	0	16	29	1	46	583
30-45	21	91	93	0	205	0	71	71	26	168	35	17	32	0	84	0	21	27	6	54	511
45-00	14	122	109	1	246	0	101	101	23	225	45	14	23	0	82	0	17	33	4	54	607
09:0-15	18	83	54	0	155	0	68	68	18	154	40	23	12	0	75	0	15	27	2	44	428
15-30	9	63	60	0	132	0	94	94	12	200	39	16	16	0	71	0	9	31	3	43	446
3 Hr Totals	153	1052	1177	4	2386	0	784	784	291	1859	368	182	187	0	737	1	134	333	22	490	4598
1 Hr Totals																					
630-730	29	305	326	0	660	0	163	163	91	417	72	46	39	0	157	1	22	94	2	119	1353
645-745	37	332	381	2	752	0	198	198	110	506	83	51	50	0	184	1	28	108	3	140	1582
07-08	51	344	457	2	854	0	233	233	108	574	98	54	56	0	208	1	33	123	5	162	1798
715-815	57	360	510	3	930	0	261	261	119	641	122	61	57	0	240	1	39	125	5	170	1981
730-830	62	388	535	3	988	0	287	287	121	695	137	66	65	0	268	0	50	121	5	176	2127
745-845	73	401	503	1	978	0	294	294	115	703	147	70	78	0	295	0	59	115	10	184	2160
08-09	70	424	474	2	970	0	313	313	113	739	158	72	89	0	319	0	69	119	12	200	2228
815-915	68	413	397	1	879	0	320	320	100	740	155	73	84	0	312	0	69	116	13	198	2129
830-930	62	359	316	1	738	0	334	334	79	747	159	70	83	0	312	0	62	118	15	195	1992
PEAK HOUR																					
08-09	70	424	474	2	970	0	313	313	113	739	158	72	89	0	319	0	69	119	12	200	2228
PM																					
04:0-15	14	61	60	0	135	1	3	109	20	133	84	42	25	0	151	0	25	21	9	55	474
15-30	17	82	38	0	137	0	3	83	8	94	86	53	41	0	180	0	25	18	5	48	459
30-45	24	84	30	0	138	0	5	108	9	122	100	43	34	0	177	0	30	21	8	59	496
45-00	38	91	37	0	166	0	6	92	15	113	98	57	55	0	210	0	29	22	11	62	551
05:0-15	19	103	54	0	176	0	3	136	26	165	104	46	51	0	201	0	39	19	9	67	609
15-30	28	134	59	0	221	0	3	127	19	149	126	63	56	0	245	0	35	29	5	69	684
30-45	27	104	45	0	176	0	3	123	25	151	120	50	45	0	215	0	29	25	9	63	605
45-00	24	113	76	0	213	0	3	132	14	149	132	53	55	0	240	0	37	21	7	65	667
06:0-15	29	88	58	0	175	0	5	110	20	135	112	49	63	0	224	0	53	27	11	91	625
15-30	32	103	62	0	197	0	6	119	20	145	103	78	61	0	242	0	43	29	11	83	667
30-45	23	103	58	0	184	0	6	121	17	144	102	65	42	0	209	0	44	30	14	88	667
45-00	30	90	43	0	163	0	5	88	19	112	102	63	50	0	215	0	49	26	4	79	569
3 Hr Totals	305	1156	620	0	2081	1	51	1348	212	1612	1269	662	578	0	2509	0	438	288	103	829	7031
1 Hr Totals																					
04-05	93	318	165	0	576	1	17	392	52	462	368	195	155	0	718	0	109	82	33	224	1980
415-515	98	360	159	0	617	0	17	419	58	494	388	199	181	0	768	0	123	80	33	236	2115
430-530	109	412	180	0	701	0	17	463	69	549	428	209	196	0	833	0	133	91	33	257	2340
445-545	112	432	195	0	739	0	15	478	85	578	448	216	207	0	871	0	132	95	34	261	2449
05-06	98	454	234	0	786	0	12	518	84	614	482	212	207	0	901	0	140	94	30	264	2565
515-615	108	439	238	0	785	0	14	492	78	584	490	215	219	0	924	0	154	102	32	288	2581
530-630	112	408	241	0	761	0	17	484	79	580	467	230	224	0	921	0	162	102	38	302	2564
545-645	108	407	254	0	769	0	20	482	71	573	449	245	221	0	915	0	177	107	43	327	2584
06-07	114	384	221	0	719	0	22	438	76	536	419	255	216	0	890	0	189	112	40	341	2486
545-645	108	407	254	0	769	0	20	482	71	573	449	245	221	0	915	0	177	107	43	327	2584

PEDESTRIAN OBSERVATIONS

 Intersection of: Muddy Branch Rd.
 and: Diamondback Dr. / Shop. Ctr. Acces:
 Location: Montgomery Co., Md.

 Counted by: FC, TP
 Date: November 10, 2004
 Weather: Fair, Cold
 Entered by: TT

Day: Wednesday



TIME	Muddy Branch Rd.				Muddy Branch Rd.			
	NORTH LEG				SOUTH LEG			
	ADULT		CHILDREN		ADULT		CHILDREN	
PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	
AM								
06:30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	2	0	0	0
07:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
08:0-15	1	1	1	0	3	0	0	0
15-30	0	0	0	0	1	0	0	0
30-45	0	0	0	0	1	0	0	0
45-00	1	0	0	0	1	0	0	0
09:0-15	1	0	0	0	0	0	0	0
15-30	1	0	0	0	4	0	0	0
TOTALS	4	1	1	0	1	0	1	0
PM								
04:0-15	4	0	0	0	0	0	0	0
15-30	1	0	0	0	4	0	0	0
30-45	2	0	0	0	2	0	0	0
45-00	2	0	0	0	2	0	1	0
05:0-15	1	0	0	0	0	0	0	0
15-30	2	0	0	0	1	0	0	0
30-45	1	0	0	0	1	0	0	0
45-00	0	0	0	0	0	0	0	0
06:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	3	0	0	0
45-00	0	0	0	0	0	0	0	1
TOTALS	13	0	0	0	13	0	1	1

TIME	Diamondback Dr.				Shop. Ctr. Access			
	EAST LEG				WEST LEG			
	ADULT		CHILDREN		ADULT		CHILDREN	
PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	
AM								
06:30-45	1	0	0	0	0	0	1	0
45-00	0	0	0	0	0	0	0	0
07:0-15	0	0	0	0	2	1	0	0
15-30	0	0	0	0	2	0	0	0
30-45	1	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	0
08:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	1	0	0	1	0	0	0
09:0-15	0	0	0	0	1	0	0	0
15-30	0	0	0	0	3	0	0	0
TOTALS	2	1	0	0	9	1	1	0
PM								
04:0-15	0	0	0	0	0	0	0	0
15-30	1	0	0	0	1	1	0	0
30-45	4	0	0	0	3	0	0	0
45-00	0	0	0	0	0	0	0	0
05:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	1	0	0	0
30-45	4	0	0	0	0	0	0	0
45-00	1	0	0	0	2	0	0	0
06:0-15	0	0	0	0	4	0	0	0
15-30	0	0	0	0	0	1	0	0
30-45	0	0	0	0	1	0	0	0
45-00	5	0	0	0	2	0	0	0
TOTALS	15	0	0	0	14	2	0	0

VEHICLE TURNING MOVEMENT COUNT - SUMMARY

Intersection of: Diamondabck Dr.
and: Story Dr. / Bickerstaff Way
Location: Montgomery Co., Md.

Counted by: TT

Date: November 10, 2004

Day: Wednesday

Weather: Fair, Cold

Entered by: TT



TIME	TRAFFIC FROM NORTH on: Story Dr.					TRAFFIC FROM SOUTH on: Bickerstaff Way					TRAFFIC FROM EAST on: Diamondabck Dr.					TRAFFIC FROM WEST on: Diamondabck Dr.					TOTAL N + S + E + W
	RIGHT	THRU	LEFT	U-TN	TOTAL	U-TN	LEFT	THRU	RIGHT	TOTAL	RIGHT	THRU	LEFT	U-TN	TOTAL	U-TN	LEFT	THRU	RIGHT	TOTAL	
	AM																				
06:30-45	2	0	67	0	69	0	1	1	35	37	10	25	2	0	37	0	3	101	0	104	247
45-00	8	0	54	0	62	0	0	0	25	25	5	39	1	0	45	0	0	111	1	112	244
07:0-15	12	1	78	0	91	0	0	3	40	43	5	35	4	0	44	0	3	132	0	135	313
15-30	5	0	71	0	76	0	5	2	29	36	9	39	6	0	54	0	3	154	2	159	325
30-45	5	8	87	0	100	0	2	2	40	44	10	45	10	0	65	0	7	177	1	185	394
45-00	8	0	60	0	68	0	1	1	17	19	10	45	2	0	57	0	2	181	1	184	328
08:0-15	10	2	93	0	105	0	1	0	35	36	15	77	5	1	98	0	3	181	1	185	424
15-30	12	2	74	0	88	0	3	5	48	56	14	65	6	0	85	0	9	206	1	216	445
30-45	23	0	114	0	137	0	0	8	38	46	16	66	6	1	89	0	10	143	3	156	428
45-00	28	3	86	0	117	0	2	3	28	33	12	64	2	0	78	0	5	118	1	124	352
09:0-15	15	0	106	0	121	0	3	0	33	36	10	54	13	0	77	0	7	106	3	116	350
15-30	4	2	53	0	59	0	1	0	29	30	14	55	3	0	72	0	4	107	0	111	272
3 Hr Totals	132	18	943	0	1093	0	19	25	397	441	130	609	60	2	801	0	56	1717	14	1787	3500
1 Hr Totals																					
630-730	27	1	270	0	298	0	6	6	129	141	29	138	13	0	180	0	9	498	3	510	1129
645-745	30	9	290	0	329	0	7	7	134	148	29	158	21	0	208	0	13	574	4	591	1276
07-08	30	9	296	0	335	0	8	8	126	142	34	164	22	0	220	0	15	644	4	663	1360
715-815	28	10	311	0	349	0	9	5	121	135	44	206	23	1	274	0	15	693	5	713	1471
730-830	35	12	314	0	361	0	7	8	140	155	49	232	23	1	305	0	21	745	4	770	1591
745-845	53	4	341	0	398	0	5	14	138	157	55	253	19	2	329	0	24	711	6	741	1625
08-09	73	7	367	0	447	0	6	16	149	171	57	272	19	2	350	0	27	648	6	681	1649
PEAK HOUR	78	5	380	0	463	0	8	16	147	171	52	249	27	1	329	0	31	573	8	612	1575
08-09	73	7	367	0	447	0	6	16	149	171	57	272	19	2	350	0	27	648	6	681	1649
PM																					
04:0-15	6	3	26	0	35	0	0	5	11	16	50	176	26	0	252	0	11	62	4	77	380
15-30	8	1	33	0	42	0	0	0	4	4	28	164	24	0	216	1	10	48	3	62	324
30-45	16	0	24	1	41	0	9	1	13	23	28	189	23	0	240	1	21	59	6	87	391
45-00	13	1	28	0	42	0	2	2	12	16	33	186	33	0	252	0	9	61	5	75	385
05:0-15	15	3	57	0	75	0	6	2	17	25	40	214	34	0	288	0	13	86	2	101	489
15-30	28	1	47	0	76	0	2	0	17	19	56	222	23	1	302	0	11	94	1	106	503
30-45	12	2	62	0	76	0	6	4	21	31	63	241	33	0	337	0	19	83	3	105	549
45-00	22	0	32	0	54	0	2	1	16	19	41	251	36	0	328	0	10	101	1	112	513
06:0-15	8	2	45	0	55	0	2	3	14	19	61	239	41	1	342	0	13	93	3	109	525
15-30	7	1	40	0	48	0	4	4	19	27	54	241	31	1	327	0	22	92	1	115	517
30-45	9	2	66	0	77	0	1	2	13	16	40	236	36	0	312	0	6	93	0	99	504
45-00	8	1	54	0	63	0	2	3	10	15	38	231	29	0	298	0	22	72	1	95	471
3 Hr Totals	152	17	514	1	684	0	36	27	167	230	532	2590	369	3	3494	2	167	944	30	1143	5551
1 Hr Totals																					
04-05	43	5	111	1	160	0	11	8	40	59	139	715	106	0	960	2	51	230	18	301	1480
415-515	52	5	142	1	200	0	17	5	46	68	129	753	114	0	996	2	53	254	16	325	1589
430-530	72	5	156	1	234	0	19	5	59	83	157	811	113	1	1082	1	54	300	14	369	1768
445-545	68	7	194	0	269	0	16	8	67	91	192	863	123	1	1179	0	52	324	11	387	1926
05-06	77	6	198	0	281	0	16	7	71	94	200	928	126	1	1255	0	53	364	7	424	2054
515-615	70	5	186	0	261	0	12	8	68	88	221	953	133	2	1309	0	53	371	8	432	2090
530-630	49	5	179	0	233	0	14	12	70	96	219	972	141	2	1334	0	64	369	8	441	2104
545-645	46	5	183	0	234	0	9	10	62	81	196	967	144	2	1309	0	51	379	5	435	2059
06-07	32	6	205	0	243	0	9	12	56	77	193	947	137	2	1279	0	63	350	5	418	2017
530-630	49	5	179	0	233	0	14	12	70	96	219	972	141	2	1334	0	64	369	8	441	2104

PEDESTRIAN OBSERVATIONS

 Intersection of: Diamondabck Dr.
 and: Story Dr. / Bickerstaff Way
 Location: Montgomery Co., Md.

 Counted by: TT
 Date: November 10, 2004
 Weather: Fair, Cold
 Entered by: TT

Day: Wednesday



TIME	Story Dr.				Bickerstaff Way			
	NORTH LEG				SOUTH LEG			
	ADULT		CHILDREN		ADULT		CHILDREN	
	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES
AM								
06:30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	1	0
07:0-15	0	0	0	0	2	0	0	0
15-30	0	0	0	0	1	0	0	0
30-45	0	0	0	0	5	0	0	0
45-00	0	0	0	0	2	0	0	0
08:0-15	1	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	2	0	0	0
09:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
TOTALS	1	0	0	0	12	0	1	0
PM								
04:0-15	1	0	0	0	1	0	0	0
15-30	0	0	0	0	1	0	0	0
30-45	0	0	0	0	4	0	0	0
45-00	1	0	0	0	2	0	0	0
05:0-15	2	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	2	0
45-00	1	0	0	0	1	0	1	0
06:0-15	0	0	0	0	1	0	0	0
15-30	1	0	0	0	0	0	0	0
30-45	4	0	1	0	1	0	0	0
45-00	0	0	0	0	0	0	0	0
TOTALS	10	0	1	0	11	0	3	0

TIME	Diamondabck Dr.				Diamondabck Dr.			
	EAST LEG				WEST LEG			
	ADULT		CHILDREN		ADULT		CHILDREN	
	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES	PEDESTRIANS	BICYCLES
AM								
06:30-45	0	0	0	0	1	0	0	0
45-00	0	0	0	0	0	1	0	2
07:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	1	1	1
45-00	0	0	0	0	1	0	0	0
08:0-15	1	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	0	0	0	0
45-00	0	0	0	0	0	0	0	1
09:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	1	0	0	0
TOTALS	1	0	0	0	4	2	1	4
PM								
04:0-15	1	0	0	0	1	0	0	1
15-30	2	0	0	0	1	0	0	0
30-45	1	0	0	0	0	0	0	0
45-00	0	0	0	0	1	0	1	0
05:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	0	0	0	0	1	0	0	0
45-00	0	0	0	0	0	0	0	0
06:0-15	0	0	0	0	0	0	0	0
15-30	0	0	0	0	0	0	0	0
30-45	1	0	0	0	0	0	0	0
45-00	0	0	0	0	4	1	0	0
TOTALS	5	0	0	0	8	1	1	1

APPENDIX B

*Intersection Capacity
Analysis Worksheets*



CRITICAL LANE VOLUME (CLV) METHODOLOGY
for Montgomery County

sli_0410111.xls-clv, 11/23/04

Intersection of: MD 119
and: Sam Eig Hwy
Conditions: 2004 Existing Traffic

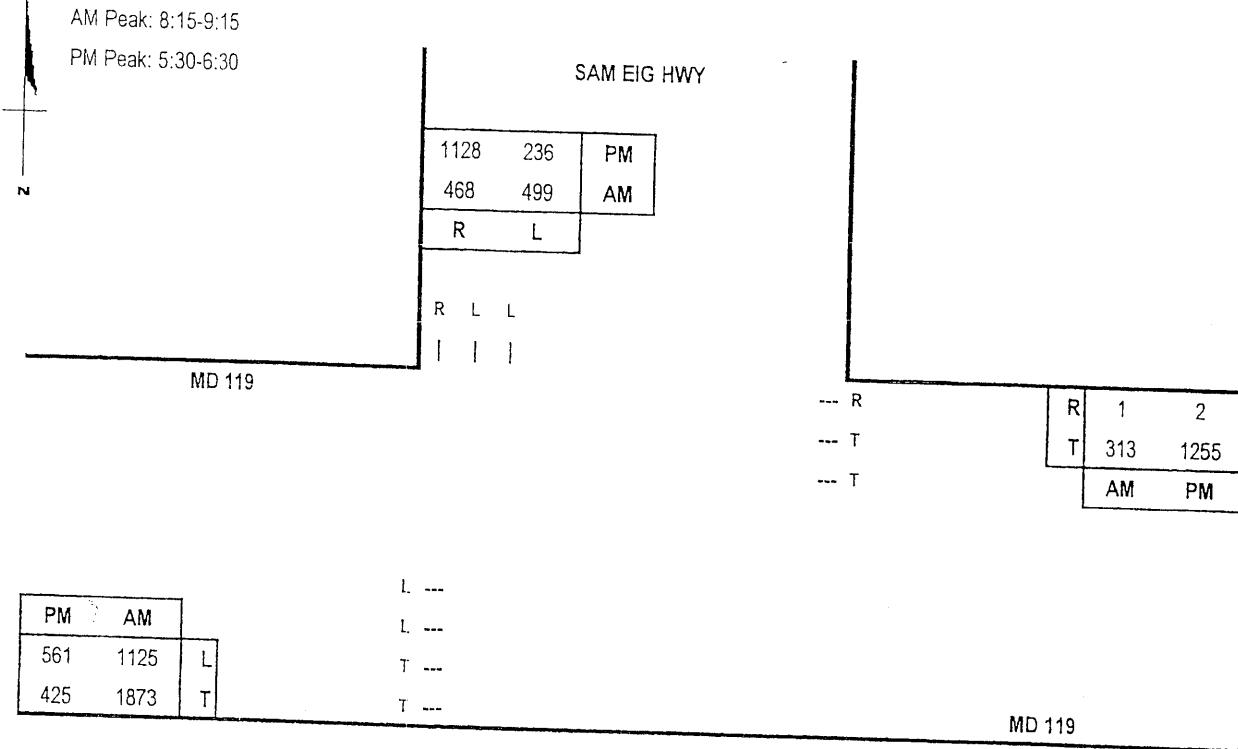
Date of Count: 11/4/2004
Day of Week: Thursday
Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 8:15-9:15

PM Peak: 5:30-6:30



Capacity Analysis

Morning Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		AM CLV
	VOL	x LUF	= Total	VOL	x LJF	
SB	499	0.53	264			264
EB	1873	0.55	1030			1030
WB	313	0.53	166	1125	0.53	596
				CLV TOTAL=	1294	
				Level of Service (LOS)=	C	
				AM V/C =	0.81	

Scenario ID - EXIST1

Evening Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		PM CLV
	VOL	x LUF	= Total	VOL	x LJF	
SB	831	1.00	831			831
EB	425	0.55	234			962
WB	1255	0.53	665	561	0.53	297
				CLV TOTAL=	1793	
				Level of Service (LOS)=	F	
				PM V/C =	1.12	

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

Intersection of: MD 119

Date of Count: 11/4/2004

and: Sam Eig Hwy

Day of Week: Thursday

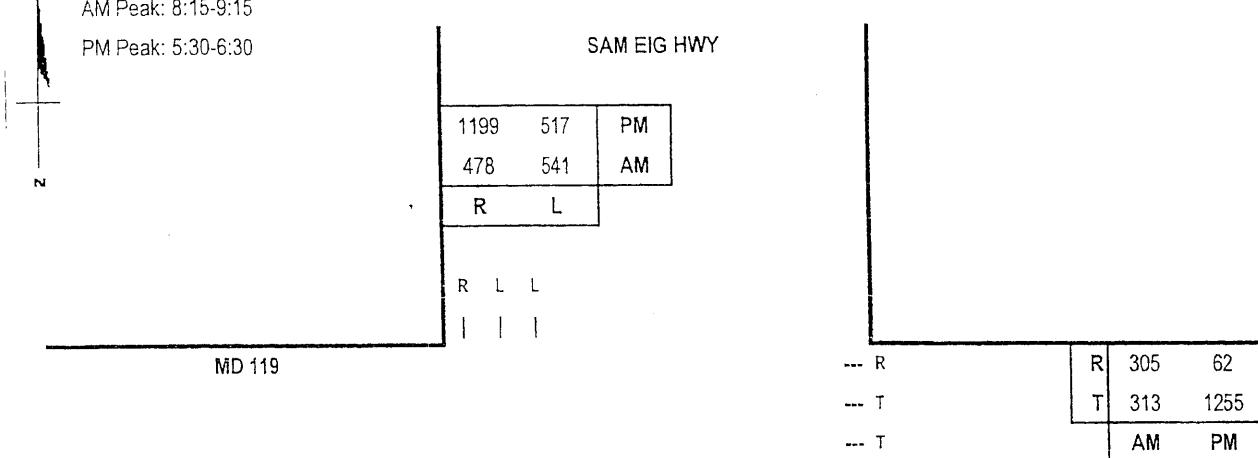
Conditions: Background Traffic

Analyst: Shulin Li

Lane Use + Traffic Volumes

AM Peak: 8:15-9:15

PM Peak: 5:30-6:30

**Capacity Analysis**

Morning Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		AM CLV
	VOL	x LUF	= Total	VOL	x LUF	
SB	541	0.53	287			287
EB	1873	0.55	1030			1030
WB	313	0.53	166	1201	0.53	637
CLV TOTAL=						1317
Level of Service (LOS) =						D

Scenario ID - BACK1

AM V/C = 0.82

Evening Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		PM CLV
	VOL	x LUF	= Total	VOL	x LUF	
SB	894	1.00	894			894
EB	425	0.55	234			970
WB	1255	0.53	665	576	0.53	305
CLV TOTAL=						1864
Level of Service (LOS) =						F

PM V/C = 1.17

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

Intersection of: MD 119

Date of Count: 11/4/2004

and: Sam Eig Hwy

Day of Week: Thursday

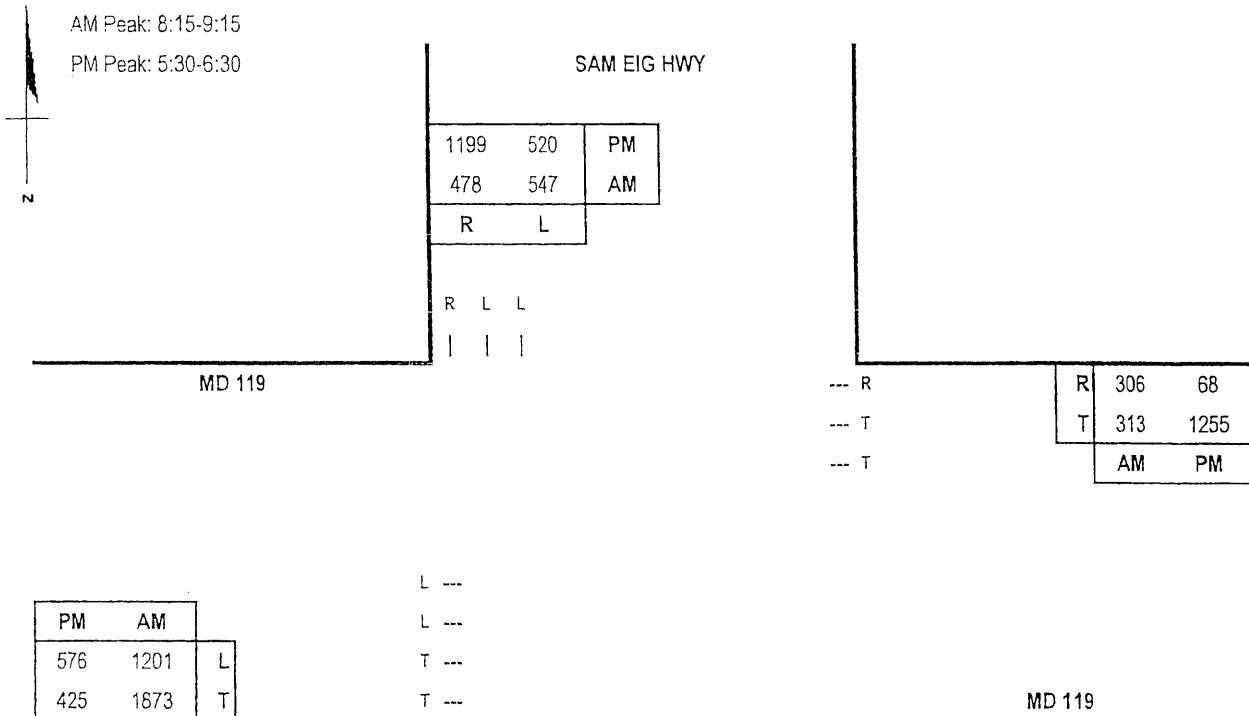
Conditions: Total Traffic

Analyst: Shulin Li

Lane Use + Traffic Volumes

AM Peak: 8:15-9:15

PM Peak: 5:30-6:30

Capacity Analysis

Morning Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		AM CLV
	VOL	x LUF	= Total	VOL	x LUF	
SB	547	0.53	290			290
EB	1873	0.55	1030			1030
WB	313	0.53	166	1201	0.53	637
CLV TOTAL =				1320		
Level of Service (LOS) = D						

Scenario ID - TOT1

AM V/C = 0.83

Evening Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		PM CLV
	VOL	x LUF	= Total	VOL	x LUF	
SB	894	1.00	894			894
EB	425	0.55	234			970
WB	1255	0.53	665	576	0.53	305
CLV TOTAL =				1864		
Level of Service (LOS) = F						
PM V/C = 1.17						

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for MSHA

sli_0410111.xls-clv, 11/23/04

Intersection of: MD 119
and: Sam Eig Hwy
Conditions: Total Traffic

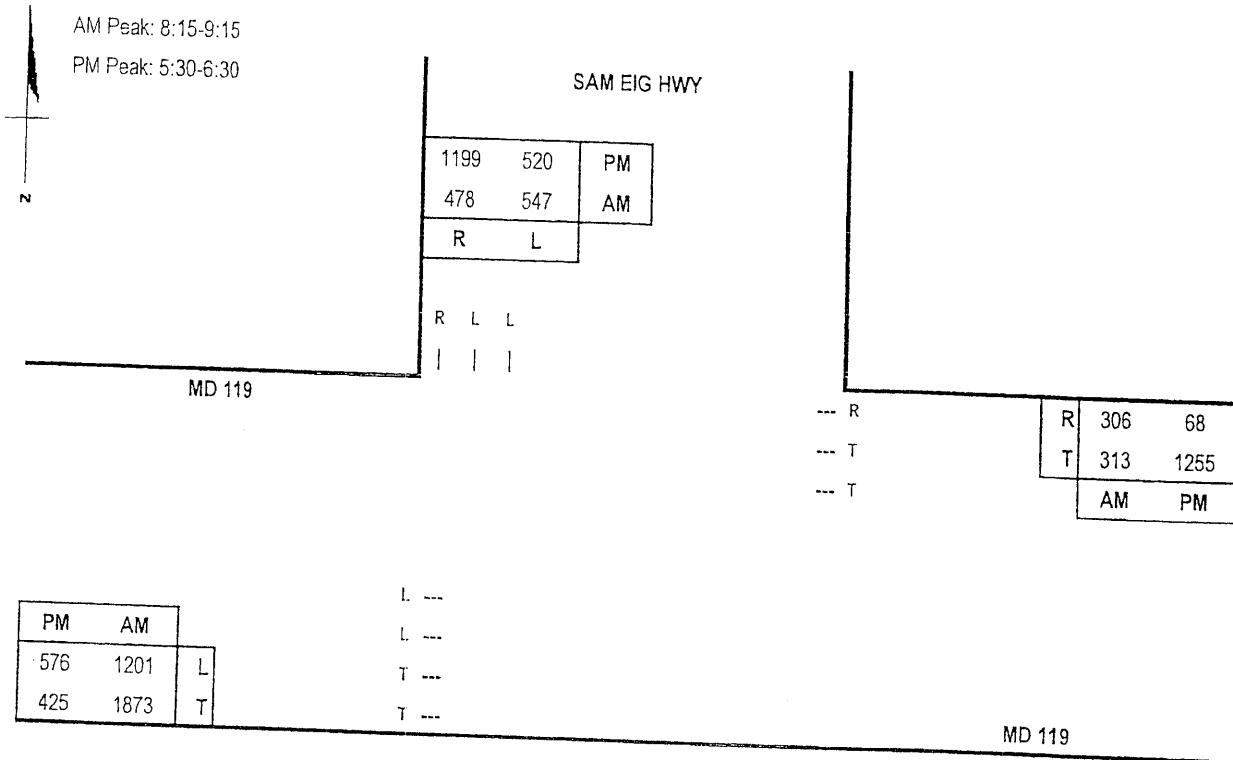
Date of Count: 11/4/2004
Day of Week: Thursday
Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 8:15-9:15

PM Peak: 5:30-6:30



Capacity Analysis

Morning Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		AM CLV
	VOL	x LUF	= Total	VOL	x LUF	
SB	547	0.60	328			328
EB	1873	0.55	1030			1030
WB	313	0.55	172	1201	0.60	721
CLV TOTAL =				1358		
Level of Service (LOS) = D						
AM V/C = 0.85						

Scenario ID - TOT1

Evening Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		PM CLV
	VOL	x LUF	= Total	VOL	x LUF	
SB	853	1.00	853			853
EB	425	0.55	234			
WB	1255	0.55	690	576	0.60	346
CLV TOTAL =				1889		
Level of Service (LOS) = F						
PM V/C = 1.18						

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

Intersection of: MD 119

Date of Count: 11/4/2004

and: Sam Eig Hwy

Day of Week: Thursday

Conditions: Background Traffic

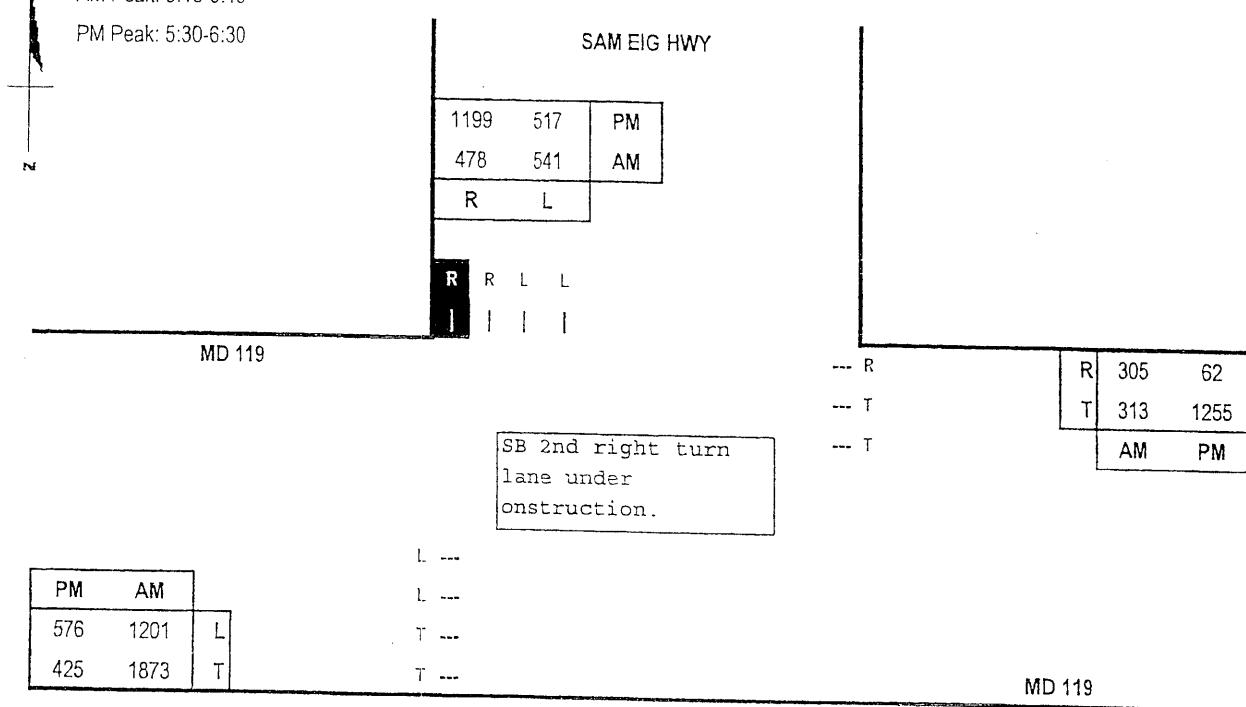
Analyst: Shulin Li

w/ improvement

Lane Use + Traffic Volumes

AM Peak: 8:15-9:15

PM Peak: 5:30-6:30

Capacity Analysis

Morning Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		AM CLV
	VOL	x LUF	= Total	VOL	x LUF	
SB	541	0.53	287			287
EB	1873	0.55	1030			1030
WB	313	0.53	166	1201	0.53	637
CLV TOTAL =				1317		
Level of Service (LOS) =				D		

Scenario ID - BACK1

AM V/C = 0.82

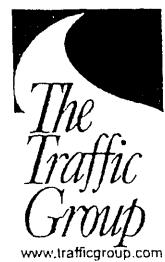
Evening Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		PM CLV
	VOL	x LUF	= Total	VOL	x LUF	
SB	623	0.53	330			330
EB	425	0.55	234			
WB	1255	0.53	665	576	0.53	305
CLV TOTAL =				1300		
Level of Service (LOS) =				C		
PM V/C = 0.81						

CRITICAL LANE VOLUME (CLV) METHODOLOGY

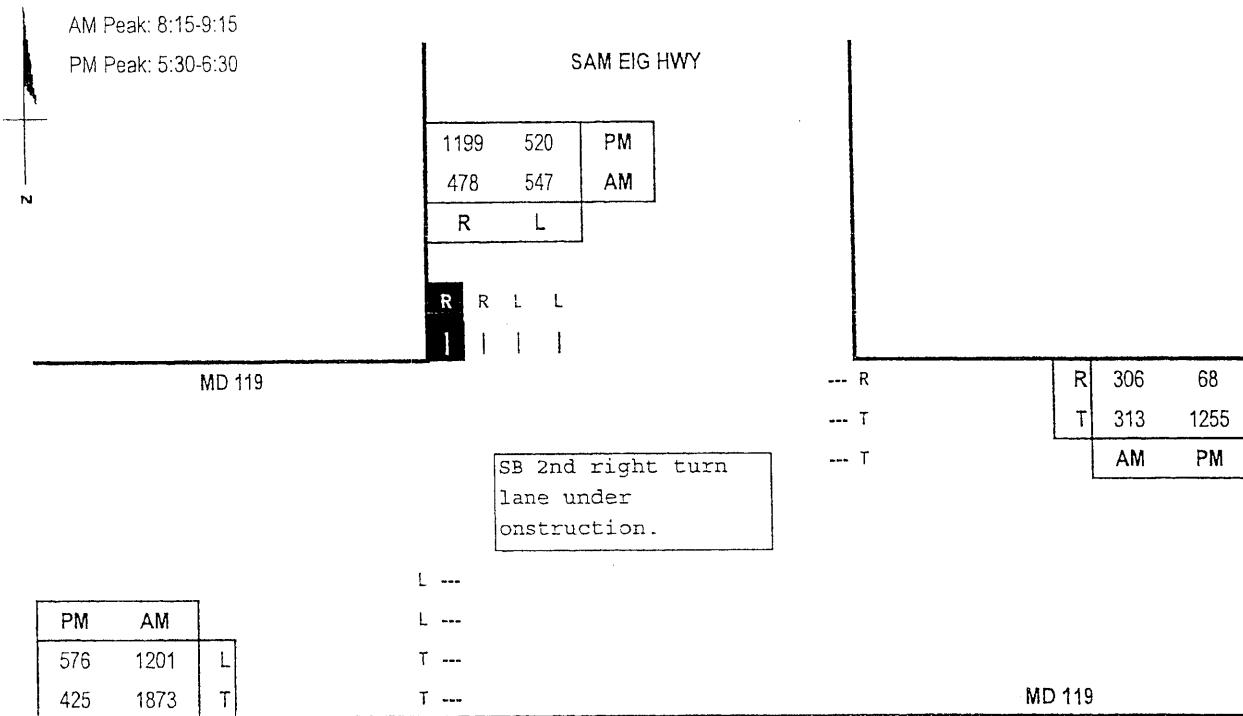
for Montgomery County

Intersection of: MD 119
and: Sam Eig Hwy
Conditions: Total Traffic
w/ improvement

Date of Count: 11/4/2004
Day of Week: Thursday
Analyst: Shulin Li



Lane Use + Traffic Volumes



Capacity Analysis

Morning Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		AM CLV
	VOL	x LUF	= Total	VOL	x LUF	
SB	547	0.53	290			290
EB	1873	0.55	1030			1030
WB	313	0.53	166	1201	0.53	637
CLV TOTAL =				1320		
Level of Service (LOS) =				D		

Scenario ID - TOT1

AM V/C = 0.83

Evening Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		PM CLV
	VOL	x LUF	= Total	VOL	x LUF	
SB	623	0.53	330			330
EB	425	0.55	234			970
WB	1255	0.53	665	576	0.53	305
CLV TOTAL =				1300		
Level of Service (LOS) =				C		

PM V/C = 0.81

CRITICAL LANE VOLUME (CLV) METHODOLOGY for MSHA

sli_041011\1i.xls-clv, 11/23/04

Intersection of: MD 119
and: Sam Eig Hwy
Conditions: Total Traffic
w/ improvement

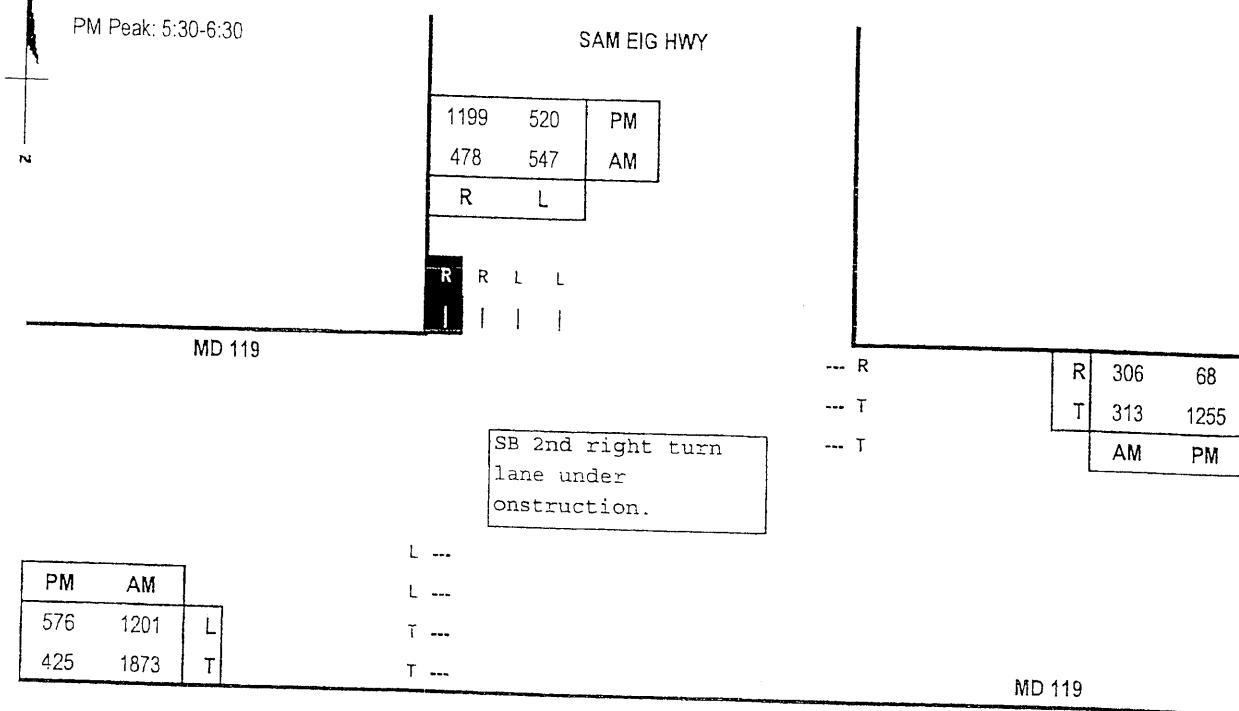
Date of Count: 11/4/2004
Day of Week: Thursday
Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 8:15-9:15

PM Peak: 5:30-6:30



Capacity Analysis

Morning Peak Hour					
Dir	Thru Volumes			+ Opposing Lefts	
	VOL	x LUF	= Total	VOL	x LUF = Total
SB	547	0.60	328		328
EB	1873	0.55	1030		1030
WB	313	0.55	172	1201	0.60 721
			CLV TOTAL=	1358	
Level of Service (LOS)=			D		
AM V/C =0.85					

Evening Peak Hour					
Dir	Thru Volumes			+ Opposing Lefts	
	VOL	x LUF	= Total	VOL	x LUF = Total
SB	571	0.55	314		314
EB	425	0.55	234		1036
WB	1255	0.55	690	576	0.60 346
			CLV TOTAL=	1350	
Level of Service (LOS)=			D		
AM V/C =0.85					
PM V/C =0.84					

Scenario ID - TOT1

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

Intersection of: Sam EIG Hwy

and: Diamondback Dr

Conditions: 2004 Existing Traffic

Date of Count: 11/3/2004

Day of Week: Wednesday

Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 7:45-8:45

PM Peak: 5:15-6:15

SAM EIG HWY

1244	1179	4	PM
305	883	4	AM
R	T	U	

R R T T
| | | |

DIAMONDBACK DR

PM	AM	
477	959	L
0	0	T
7	29	R

L ...
L ...
R ...

| | |
LT T T

AM	L	T	R
25	1558	0	

PM	L	T	R
74	1277	0	

SAM EIG HWY

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			AM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	1583	0.37	586	4	0.00	0	586
SB	1192	0.37	441	25	1.00	25	
EB	4	1.00	4	0	0.00	0	508
WB	0	0.00	0	959	0.53	508	
CLV TOTAL =				1094			
Level of Service (LOS) =				B			
AM V/C =				0.68			

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			PM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	1351	0.37	500	4	0.00	0	
SB	995	1.00	995	74	1.00	74	1069
EB	0	0.00	0	0	0.00	0	
WB	0	0.00	0	477	0.53	253	253
CLV TOTAL =				1322			
Level of Service (LOS) =				D			
PM V/C =				0.83			

Scenario ID - EXIST2

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

sli_0410112.xls-clv, 11/23/04

Intersection of: Sam Eig Hwy
and: Diamondback Dr
Conditions: Background Traffic

Date of Count: 11/3/2004
Day of Week: Wednesday
Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 7:45-8:45

PM Peak: 5:15-6:15

SAM EIG HWY

1300	1531	4	PM
315	935	4	AM
R	T	U	

R RT T T
| | | |

DIAMONDBACK DR

PM	AM	
489	1019	L
0	0	T
7	29	R

L ...
L ...
R ...

| | |
LT T T

	L	T	R
AM	25	1938	0
PM	74	1352	0

SAM EIG HWY

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			AM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	1963	0.37	726	4	0.00	0	726
SB	1254	0.37	464	25	1.00	25	
EB	4	1.00	4	0	0.00	0	540
WB	0	0.00	0	1019	0.53	540	
CLV TOTAL =				1266			
Level of Service (LOS) =				C			
AM V/C = 0.79							

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			PM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	1426	0.37	528	4	0.00	0	
SB	2835	0.37	1049	74	1.00	74	1123
EB	0	0.00	0	0	0.00	0	
WB	0	0.00	0	489	0.53	259	259
CLV TOTAL =				1382			
Level of Service (LOS) =				D			
PM V/C = 0.86							

Scenario ID - BACK2

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

sli_041011\2.xls-clv, 11/23/04

Intersection of: Sam Eig Hwy
and: Diamondback Dr
Conditions: Total Traffic

Date of Count: 11/3/2004
Day of Week: Wednesday
Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 7:45-8:45

PM Peak: 5:15-6:15

SAM EIG HWY

1319	1531	4	PM
320	935	4	AM
R	T	U	

R RT T T

| | | |

DIAMONDBACK DR

PM	AM	
500	1039	L
0	0	T
10	35	R

L ---
L ---
R ---

| | |
LT T T

	L	T	R
AM	26	1938	0
PM	80	1352	0

SAM EIG HWY

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			AM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	1964	0.37	727	4	0.00	0	727
SB	1259	0.37	466	26	1.00	26	
EB	9	1.00	9	0	0.00	0	551
WB	0	0.00	0	1039	0.53	551	
CLV TOTAL =				1278			
Level of Service (LOS) =				C			
AM V/C = 0.8							

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			PM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	1432	0.37	530	4	0.00	0	
SB	2854	0.37	1056	80	1.00	80	1136
EB	0	0.00	0	0	0.00	0	
WB	0	0.00	0	500	0.53	265	265
CLV TOTAL =				1401			
Level of Service (LOS) =				D			
PM V/C = 0.88							

Scenario ID - TOT2

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

sli_041011\3.xls-clv, 11/23/04

Intersection of: Sam Eig Hwy

Date of Count: 11/3/2004

and: Fields Rd

Day of Week: Wednesday

Conditions: 2004 Existing Traffic

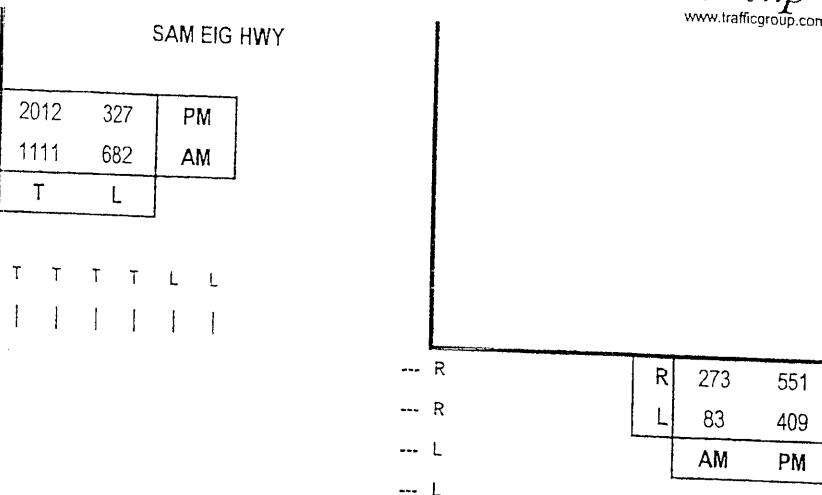
Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 7:45-8:45

PM Peak: 5:00-6:00



Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts		AM CLV
	VOL	x LUF	= Total	VOL	x LUF	
WB	83	0.53	44			44
NB	2057	0.37	761	682	0.53	361
SB	1111	0.30	333			1122
CLV TOTAL =				1166		
Level of Service (LOS) =				C		
AM V/C = 0.73						

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts		PM CLV
	VOL	x LUF	= Total	VOL	x LUF	
WB	409	0.53	217			217
NB	1420	0.37	525	327	0.53	173
SB	2012	0.30	604			698
CLV TOTAL =				915		
Level of Service (LOS) =				A		
PM V/C = 0.57						

Scenario ID - EXIST3

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

sli_041011\3.xls-clv, 11/23/04

Intersection of: Sam Eig Hwy

Date of Count: 11/3/2004

and: Fields Rd

Day of Week: Wednesday

Conditions: Background Traffic

Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 7:45-8:45

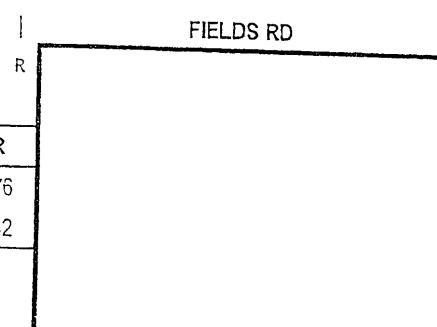
PM Peak: 5:00-6:00

SAM EIG HWY

2313	365	PM
1156	870	AM
T	L	

T T T T L L
| | | | | |

---	R	R	299	722
---	R	L	100	516
---	L			
---	L		AM	PM



SAM EIG HWY

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			AM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
WB	100	0.53	53				53
NB	2380	0.37	881	870	0.53	461	1342
SB	1156	0.30	347				

CLV TOTAL= 1395

Level of Service (LOS)= D

AM V/C = 0.87

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			PM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
WB	516	0.53	273				273
NB	1484	0.37	549	365	0.53	193	742
SB	2313	0.30	694				

CLV TOTAL= 1015

Level of Service (LOS)= B

PM V/C = 0.63

Scenario ID - BACK3

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

sli_041011\3.xls-clv, 11/23/04

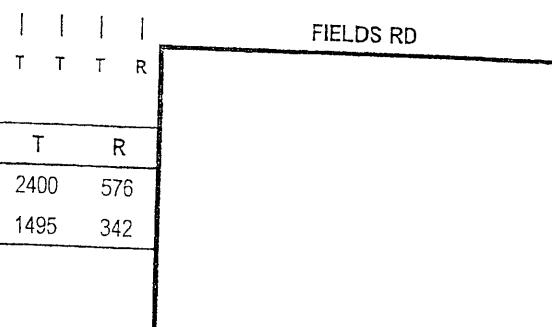
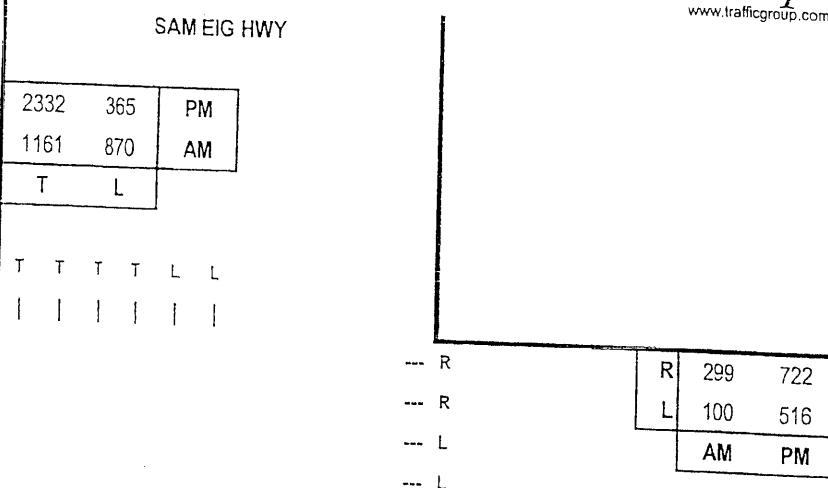
Intersection of: Sam Eig Hwy
and: Fields Rd
Conditions: Total Traffic

Date of Count: 11/3/2004
Day of Week: Wednesday
Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 7:45-8:45
PM Peak: 5:00-6:00



SAM EIG HWY

Capacity Analysis

Morning Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		AM CLV
	VOL	x LUF	= Total	VOL	x LUF	
WB	100	0.53	53			53
NB	2400	0.37	888	870	0.53	461
SB	1161	0.30	348			1349

CLV TOTAL= 1402

Level of Service (LOS)= D

AM V/C = 0.88

Evening Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		PM CLV
	VOL	x LUF	= Total	VOL	x LUF	
WB	516	0.53	273			273
NB	1495	0.37	553	365	0.53	193
SB	2332	0.30	700			746

CLV TOTAL= 1019

Level of Service (LOS)= B

PM V/C = 0.64

Scenario ID - TOT3

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

Intersection of: MD 119

and: Muddy Branch Rd

Conditions: 2004 Existing Traffic

Date of Count: 11/10/2004

Day of Week: Wednesday

Analyst: Shulin Li

Lane Use + Traffic Volumes

AM Peak: 7:45-8:45

PM Peak: 5:30-6:30

MUDDY BRANCH RD

400	485	121	PM
127	302	191	AM
R	T	L	

FR T T L

| | | |

MD 119

---	FR	R	69	166
---	T	T	548	2140
---	T	L	124	226
---	L			
		AM	PM	

PM	AM	
256	105	L
847	2825	T
196	87	R

L ---
T ---
T ---
T ---
R ---

| | | |
L T T FR

MD 119

L	T	R	
AM	457	296	95
PM	257	351	134

MUDDY BRANCH RD

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts		AM CLV	
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	296	0.53	157	191	1.00	191	617
SB	302	0.53	160	457	1.00	457	
EB	2825	0.37	1045	124	1.00	124	1169
WB	548	0.53	290	105	1.00	105	

CLV TOTAL = 1786

Level of Service (LOS) = F

AM V/C = 1.12

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts		PM CLV	
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	351	0.53	186	121	1.00	121	
SB	485	0.53	257	257	1.00	257	514
EB	847	0.37	313	226	1.00	226	
WB	2140	0.53	1134	256	1.00	256	1390

CLV TOTAL = 1904

Level of Service (LOS) = F

PM V/C = 1.19

Scenario ID - EXIST4

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

sli_041011\4.xls-clv, 11/23/04

Intersection of: MD 119
and: Muddy Branch Rd
Conditions: Background Traffic

Date of Count: 11/10/2004
Day of Week: Wednesday
Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 7:45-8:45

PM Peak: 5:30-6:30

MUDGY BRANCH RD

408	505	121	PM
129	305	191	AM
R	T	L	

FR T T L
| | | |

MD 119

---	FR	R	69	166
---	T	T	551	2160
---	T	L	131	277
---	L			

AM PM

L ---
T ---
T ---
R ---

| | | |
L T T FR

PM	AM	
258	113	L
852	2847	T
196	87	R

	L	T	R
AM	457	318	149
PM	257	355	144

MUDGY BRANCH RD

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			AM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	318	0.53	169	191	1.00	191	619
SB	305	0.53	162	457	1.00	457	
EB	2847	0.37	1053	131	1.00	131	1184
WB	551	0.53	292	113	1.00	113	

CLV TOTAL = 1803

Level of Service (LCS) = F

AM V/C = 1.13

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			PM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	355	0.53	188	121	1.00	121	
SB	505	0.53	268	257	1.00	257	525
EB	852	0.37	315	277	1.00	277	
WB	2160	0.53	1145	258	1.00	258	1403

CLV TOTAL = 1928

Level of Service (LOS) = F

PM V/C = 1.21

Scenario ID - BACK4

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

Intersection of: MD 119
and: Muddy Branch Rd
Conditions: Total Traffic

Date of Count: 11/10/2004
Day of Week: Wednesday
Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 7:45-8:45

PM Peak: 5:30-6:30

MUDDY BRANCH RD

408	505	121	PM
130	306	191	AM
R	T	L	

FR T T L
| | | |

MD 119

--- FR	R	69	166
--- T	T	551	2160
--- T	L	131	277
--- L		AM	PM

L ---
T ---
T ---
T ---
R ---

MD 119

| | | |
L T T FR

	L	T	R
AM	457	318	149
PM	257	356	144

MUDDY BRANCH RD

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts		AM CLV	
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	318	0.53	169	191	1.00	191	619
SB	306	0.53	162	457	1.00	457	
EB	2847	0.37	1053	131	1.00	131	1184
WB	551	0.53	292	113	1.00	113	
CLV TOTAL =				1803			
Level of Service (LOS) =				F			

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts		PM CLV	
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	356	0.53	189	121	1.00	121	525
SB	505	0.53	268	257	1.00	257	
EB	852	0.37	315	277	1.00	277	1404
WB	2160	0.53	1145	259	1.00	259	
CLV TOTAL =				1929			
Level of Service (LOS) =				F			

Scenario ID - TOT4

AM V/C = 1.13

PM V/C = 1.21

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for MSHA

sli_041011\4.xls-clv, 11/23/04

Intersection of: MD 119

and: Muddy Branch Rd

Conditions: Total Traffic

Date of Count: 11/10/2004

Day of Week: Wednesday

Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 7:45-8:45

PM Peak: 5:30-6:30

MUDGY BRANCH RD

408	505	121	PM
130	306	191	AM
R	T	L	

FR T T L
| | | |

MD 119

---	FR	R	69	166
---	T	T	551	2160
---	T	L	131	277
---	L		AM	PM

PM	AM	
259	113	L
852	2847	T
196	87	R

L ---
T ---
T ---
T ---
R ---

| | | |
L T T FR

	L	T	R
AM	457	318	149
PM	257	356	144

MUDGY BRANCH RD

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts		AM CLV	
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	318	0.55	175	191	1.00	191	
SB	306	0.55	168	457	1.00	457	625
EB	2847	0.40	1139	131	1.00	131	
WB	551	0.55	303	113	1.00	113	1270
				CLV TOTAL=	1895		
				Level of Service (LOS)=	F		
				AM V/C =	1.18		

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts		PM CLV	
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	356	0.55	196	121	1.00	121	
SB	505	0.55	278	257	1.00	257	535
EB	852	0.40	341	277	1.00	277	
WB	2160	0.55	1188	259	1.00	259	1447
				CLV TOTAL=	1982		
				Level of Service (LOS)=	F		
				PM V/C =	1.24		

Scenario ID - TOT4

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

Intersection of: MD 119

and: Muddy Branch Rd

Conditions: Background Traffic
w/ improvement

Date of Count: 11/10/2004

Day of Week: Wednesday

Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 7:45-8:45

PM Peak: 5:30-6:30

MUDGY BRANCH RD

408	505	121	PM
129	305	191	AM
R	T	L	

FR	T	T	L	1

MD 119

--- FR

--- T

--- T

--- L

R	69	166
T	551	2160
L	131	277
AM		
PM		

PM	AM	
258	113	L
852	2847	T
196	87	R

L ...
T ...
T ...
T ...
R ...

construction complete for
SB 2nd left turn lane

MD 119

AM	L	T	R
457	318	149	

MUDDY BRANCH RD

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			AM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	318	0.53	169	191	0.53	101	
SB	305	0.53	162	457	1.00	457	619
EB	2847	0.37	1053	131	1.00	131	
WB	551	0.53	292	113	1.00	113	1184

CLV TOTAL = 1803

Level of Service (LOS) = F

AM V/C = 1.13

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			PM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	355	0.53	188	121	0.53	64	
SB	505	0.53	268	257	1.00	257	525
EB	852	0.37	315	277	1.00	277	
WB	2160	0.53	1145	258	1.00	258	1403

CLV TOTAL = 1928

Level of Service (LOS) = F

PM V/C = 1.21

Scenario ID - BACK4

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

sli_0410114i.xls-clv, 11/23/04

Intersection of: MD 119

and: Muddy Branch Rd

Conditions: Total Traffic

w/ improvement

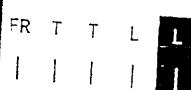
Lane Use + Traffic Volumes

AM Peak: 7:45-8:45

PM Peak: 5:30-6:30

MUDGY BRANCH RD

408	505	121	PM
130	306	191	AM
R	T	L	



MD 119

PM	AM	
259	113	L
852	2847	T
196	87	R

L ---
T ---
T ---
T ---
R ---

construction complete for
SB 2nd left turn lane

---	FR	69	166
---	T	551	2160
---	T	131	277
---	L	AM	PM

MD 119

	L	T	R
AM	457	318	149
PM	257	356	144

MUDGY BRANCH RD

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			AM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	318	0.53	169	191	0.53	101	619
SB	306	0.53	162	457	1.00	457	
EB	2847	0.37	1053	131	1.00	131	1184
WB	551	0.53	292	113	1.00	113	
CLV TOTAL =				1803			
Level of Service (LOS) =				F			
AM V/C = 1.13							

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			PM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	356	0.53	189	121	0.53	64	
SB	505	0.53	268	257	1.00	257	525
EB	852	0.37	315	277	1.00	277	
WB	2160	0.53	1145	259	1.00	259	1404
CLV TOTAL =				1929			
Level of Service (LOS) =				F			
PM V/C = 1.21							

Scenario ID - TOT4



CRITICAL LANE VOLUME (CLV) METHODOLOGY

for MSHA

sli_041011\4i.xls-clv, 11/23/04

Intersection of: MD 119
and: Muddy Branch Rd
Conditions: Total Traffic
w/ improvement

Date of Count: 11/10/2004
Day of Week: Wednesday
Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 7:45-8:45
PM Peak: 5:30-6:30

MUDGY BRANCH RD

408	505	121	PM
130	306	191	AM
R	T	L	

FR	T	T	L	L

MD 119

--- FR	R	69	166
--- T	T	551	2160
--- T	L	131	277
--- L		AM	PM

construction complete for
SB 2nd left turn lane

PM	AM	
259	113	L
852	2847	T
196	87	R

L ---
T ---
T ---
T ---
R ---

MD 119

	L	T	R
AM	457	318	149
PM	257	356	144

MUDGY BRANCH RD

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			AM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	318	0.55	175	191	0.60	115	625
SB	306	0.55	168	457	1.00	457	
EB	2847	0.40	1139	131	1.00	131	1270
WB	551	0.55	303	113	1.00	113	

CLV TOTAL = 1895

Level of Service (LOS) = F

AM V/C = 1.18

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			PM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	356	0.55	196	121	0.60	73	
SB	505	0.55	278	257	1.00	257	535
EB	852	0.40	341	277	1.00	277	
WB	2160	0.55	1188	259	1.00	259	1447

CLV TOTAL = 1982

Level of Service (LOS) = F

PM V/C = 1.24

Scenario ID - TOT4



CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

Intersection of: MD 119

and: Muddy Branch Rd

Conditions: Background Traffic
w/ improvement

Date of Count: 11/10/2004

Day of Week: Wednesday

Analyst: Shulin Li

Lane Use + Traffic Volumes

AM Peak: 7:45-8:45

PM Peak: 5:30-6:30

MUDDY BRANCH RD

408	505	121	PM
129	305	191	AM
R	T	L	

FR	T	T	L	I

MD 119

PM	AM	
258	113	L
852	2847	T
196	87	R

L ---
T ---
T ---
T ---
R ---

---	FR	R	69	166
---	T	T	551	2160
---	T	L	131	277
---	L			
		AM	PM	



	L	T	R
AM	457	318	149
PM	257	355	144

MUDDY BRANCH RD

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts		AM CLV	
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	318	0.53	169	191	0.53	101	404
SB	305	0.53	162	457	0.53	242	
EB	2847	0.37	1053	131	1.00	131	1184
WB	551	0.53	292	113	1.00	113	

CLV TOTAL = 1588

Level of Service (LOS) = E

AM V/C = 0.99

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts		PM CLV	
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	355	0.53	188	121	0.53	64	
SB	505	0.53	268	257	0.53	136	404
EB	852	0.37	315	277	1.00	277	
WB	2160	0.53	1145	258	1.00	258	1403

CLV TOTAL = 1807

Level of Service (LOS) = F

PM V/C = 1.13

Scenario ID - BACK4

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

sli_041011\4ii.xls-clv, 11/23/04

Intersection of: MD 119

and: Muddy Branch Rd

Conditions: Total Traffic

w/ improvement

Date of Count: 11/10/2004

Day of Week: Wednesday

Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 7:45-8:45

PM Peak: 5:30-6:30

MUDDY BRANCH RD

408	505	121	PM
130	306	191	AM
R	T	L	

FR	T	T	L	

MD 119

---	FR	R	69	166
---	T	T	551	2160
---	T	L	131	277
---	L		AM	PM

PM	AM	
259	113	L
852	2847	T
196	87	R

L ---
T ---
T ---
T ---
R ---

	L	T	T FR

MD 119

	L	T	R
AM	457	318	149
PM	257	356	144

MUDDY BRANCH RD

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			AM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	318	0.53	169	191	0.53	101	404
SB	306	0.53	162	457	0.53	242	
EB	2847	0.37	1053	131	1.00	131	1184
WB	551	0.53	292	113	1.00	113	
CLV TOTAL =				1588			
Level of Service (LOS) =				E			
AM V/C = 0.99							

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			PM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	356	0.53	189	121	0.53	64	404
SB	505	0.53	268	257	0.53	136	
EB	852	0.37	315	277	1.00	277	
WB	2160	0.53	1145	259	1.00	259	1404
CLV TOTAL =				1808			
Level of Service (LOS) =				F			
PM V/C = 1.13							

Scenario ID - TOT4

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for MSHA

Intersection of: MD 119

and: Muddy Branch Rd

Conditions: Total Traffic

w/ improvement

Date of Count: 11/10/2004

Day of Week: Wednesday

Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 7:45-8:45

PM Peak: 5:30-6:30

MUDGY BRANCH RD

408	505	121	PM
130	306	191	AM
R	T	L	

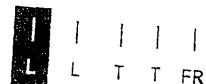
FR	T	T	L	L

MD 119

--- FR	R	69	166
--- T	T	551	2160
--- T	L	131	277
--- L		AM	PM

PM	AM	
259	113	L
852	2847	T
196	87	R

L ...
T ...
T ...
T ...
R ...



MD 119

	L	T	R
AM	457	318	149
PM	257	356	144

MUDGY BRANCH RD

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			AM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	318	0.55	175	191	0.60	115	
SB	306	0.55	168	457	0.60	274	442
EB	2847	0.40	1139	131	1.00	131	
WB	551	0.55	303	113	1.00	113	1270

CLV TOTAL = 1712

Level of Service (LOS) = F

AM V/C = 1.07

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			PM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	356	0.55	196	121	0.60	73	
SB	505	0.55	278	257	0.60	154	432
EB	852	0.40	341	277	1.00	277	
WB	2160	0.55	1188	259	1.00	259	1447

CLV TOTAL = 1879

Level of Service (LOS) = F

PM V/C = 1.17

Scenario ID - TOT4

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

Intersection of: Muddy Branch Rd
and: Diamondback Dr
Conditions: 2004 Existing Traffic

Date of Count: 11/10/2004
Day of Week: Wednesday
Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 8:00-9:00

PM Peak: 5:45-6:45

MUDGY BRANCH RD

108	407	254	PM
70	424	476	AM
R	T	L	

RT T L
| | |

SHOP CTR ACCESS

---	TR	R	158	449
---	T	T	72	245
---	L	L	89	221
		AM	AM	PM

PM	AM	
177	69	L
107	119	T
43	12	R

L ---
TR ---

	L	T	R
AM	313	313	113
PM	20	482	71

| | |
L T TR

DIAMONDBACK DR

MUDGY BRANCH RD

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			AM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	426	0.53	226	476	1.00	476	
SB	494	0.53	262	313	1.00	313	702
EB	131	1.00	131	89	1.00	89	
WB	230	0.53	122	69	1.00	69	220
				CLV TOTAL=	922		
				Level of Service (LOS) =	A		
				AM V/C =	0.58		

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			PM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	553	0.53	293	254	1.00	254	
SB	515	0.53	273	20	1.00	20	547
EB	150	1.00	150	221	1.00	221	
WB	694	0.53	368	177	1.00	177	545
				CLV TOTAL=	1092		
				Level of Service (LOS) =	B		
				PM V/C =	0.68		

Scenario ID - EXIST5

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

sli_041011\5.xls-clv, 11/23/04

Intersection of: Muddy Branch Rd
and: Diamondback Dr
Conditions: Background Traffic

Date of Count: 11/10/2004
Day of Week: Wednesday
Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 8:00-9:00
PM Peak: 5:45-6:45

MUDGY BRANCH RD

108	407	260	PM
70	424	506	AM
R	T	L	

RT T L
| | |

SHOP CTR ACCESS

--- TR	R	163	477
--- T	T	72	245
--- L	L	94	249
	AM	PM	

PM	AM	
177	69	L
107	119	T
43	12	R

L ---
TR ---

DIAMONDBACK DR

L	T	R	
AM	313	313	143
PM	20	482	77

MUDGY BRANCH RD

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts		AM CLV	
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	456	0.53	242	506	1.00	506	748
SB	494	0.53	262	313	1.00	313	
EB	131	1.00	131	94	1.00	94	225
WB	235	0.53	125	69	1.00	69	
			CLV TOTAL=	973			
Level of Service (LOS) =			A				
AM V/C = 0.61							

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts		PM CLV	
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	559	0.53	296	260	1.00	260	
SB	515	0.53	273	20	1.00	20	556
EB	150	1.00	150	249	1.00	249	
WB	722	0.53	383	177	1.00	177	560
			CLV TOTAL=	1116			
Level of Service (LOS) =			B				
PM V/C = 0.7							

Scenario ID - BACK5

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

sli_041011\5.xls-clv, 11/23/04

Intersection of: Muddy Branch Rd
and: Diamondback Dr
Conditions: Total Traffic

Date of Count: 11/10/2004
Day of Week: Wednesday
Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 8:00-9:00
PM Peak: 5:45-6:45

MUDGY BRANCH RD

108	407	261	PM
70	424	506	AM
R	T	L	

RT T L
| | |

SHOP CTR ACCESS

---	TR	R	164	477
---	T	T	72	245
---	L	L	96	249
			AM	PM

PM	AM	
177	69	L
107	119	T
43	12	R

L ---
TR ---

DIAMONDBACK DR

	L	T	R
AM	313	313	143
PM	20	482	79

MUDGY BRANCH RD

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			AM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	456	0.53	242	506	1.00	506	748
SB	494	0.53	262	313	1.00	313	
EB	131	1.00	131	96	1.00	96	227
WB	236	0.53	125	69	1.00	69	
CLV TOTAL =				975			
Level of Service (LOS) =				A			
AM V/C = 0.61							

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			PM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	561	0.53	297	261	1.00	261	
SB	515	0.53	273	20	1.00	20	558
EB	150	1.00	150	249	1.00	249	
WB	722	0.53	383	177	1.00	177	560
CLV TOTAL =				1118			
Level of Service (LOS) =				B			
PM V/C = 0.7							

Scenario ID - TOT5

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

sli_0410116.xls-clv, 11/23/04

Intersection of: Story Dr

and: Diamondabck Dr

Conditions: 2004 Existing Traffic

Date of Count: 11/10/2004

Day of Week: Wednesday

Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 8:00-9:00

PM Peak: 5:30-6:30

STORY DR

49	5	179	PM
73	7	367	AM
R	T	L	

RT L

DIAMONDABCK DR

PM	AM	
64	27	L
369	648	T
8	6	R

L ---
T ---
TR ---

AM	PM	R	57	219
		T	272	972
		L	21	143

AM PM

DIAMONDABCK DR

AM	L	T	R
14	6	16	149

BICKERSTAFF WAY

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			AM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	165	1.00	165	367	1.00	367	532
SB	80	1.00	80	6	1.00	6	
EB	654	0.53	347	21	1.00	21	368
WB	272	0.53	144	27	1.00	27	

CLV TOTAL = 900

Level of Service (LOS) = A

AM V/C = 0.56

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			PM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	82	1.00	82	179	1.00	179	
SB	54	1.00	54	14	1.00	14	261
EB	377	0.53	200	143	1.00	143	
WB	972	0.53	515	64	1.00	64	579

CLV TOTAL = 840

Level of Service (LOS) = A

PM V/C = 0.53

Scenario ID - EXIST6

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

sli_0410116.xls-clv, 11/23/04

Intersection of: Story Dr

and: Diamondabck Dr

Conditions: Background Traffic

Date of Count: 11/10/2004

Day of Week: Wednesday

Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 8:00-9:00

PM Peak: 5:30-6:30

STORY DR

49	5	179	PM
73	7	367	AM
R	T	L	

RT L

DIAMONDABCK DR

---	R	57	219
---	T	282	1028
---	T	21	143
---	L		

AM PM

PM	AM	
64	27	L
381	708	T
8	6	R

L ---
T ---
TR ---

DIAMONDABCK DR

	L	T	R
AM	6	16	149
PM	14	12	70

BICKERSTAFF WAY

Capacity Analysis

Morning Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			AM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	165	1.00	165	367	1.00	367	532
SB	80	1.00	80	6	1.00	6	
EB	714	0.53	378	21	1.00	21	399
WB	282	0.53	149	27	1.00	27	

CLV TOTAL= 931

Level of Service (LOS)= A

AM V/C = 0.58

Evening Peak Hour

Dir	Thru Volumes			+ Opposing Lefts			PM CLV
	VOL	x LUF	= Total	VOL	x LUF	= Total	
NB	82	1.00	82	179	1.00	179	
SB	54	1.00	54	14	1.00	14	261
EB	389	0.53	206	143	1.00	143	
WB	1028	0.53	545	64	1.00	64	609

CLV TOTAL= 870

Level of Service (LOS)= A

PM V/C = 0.54

Scenario ID - BACK6

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

Intersection of: Story Dr
and: Diamondabck Dr
Conditions: Total Traffic

Date of Count: 11/10/2004
Day of Week: Wednesday
Analyst: Shulin Li



Lane Use + Traffic Volumes

AM Peak: 8:00-9:00

PM Peak: 5:30-6:30

STORY DR

49	5	187	PM
75	7	381	AM
R	T	L	

RT L

DIAMONDABCK DR

---	R	60	233
---	T	282	1028
---	T	24	154
---	L		
	AM	PM	

PM	AM	
65	27	L
381	708	T
10	6	R

L ...
T ...
TR ...

	L	T	R
AM	7	16	161
PM	14	12	76

DIAMONDABCK DR

Capacity Analysis

Morning Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		AM CLV
	VOL	x LUF	= Total	VOL	x LUF	
NB	177	1.00	177	381	1.00	381
SB	82	1.00	82	7	1.00	7
EB	714	0.53	378	24	1.00	24
WB	282	0.53	149	27	1.00	27
CLV TOTAL =				960		
Level of Service (LOS) =				A		
AM V/C = 0.6						

Evening Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		PM CLV
	VOL	x LUF	= Total	VOL	x LUF	
NB	88	1.00	88	187	1.00	187
SB	54	1.00	54	14	1.00	14
EB	391	0.53	207	154	1.00	154
WB	1028	0.53	545	65	1.00	65
CLV TOTAL =				885		
Level of Service (LOS) =				A		
PM V/C = 0.55						

Scenario ID - TOT6



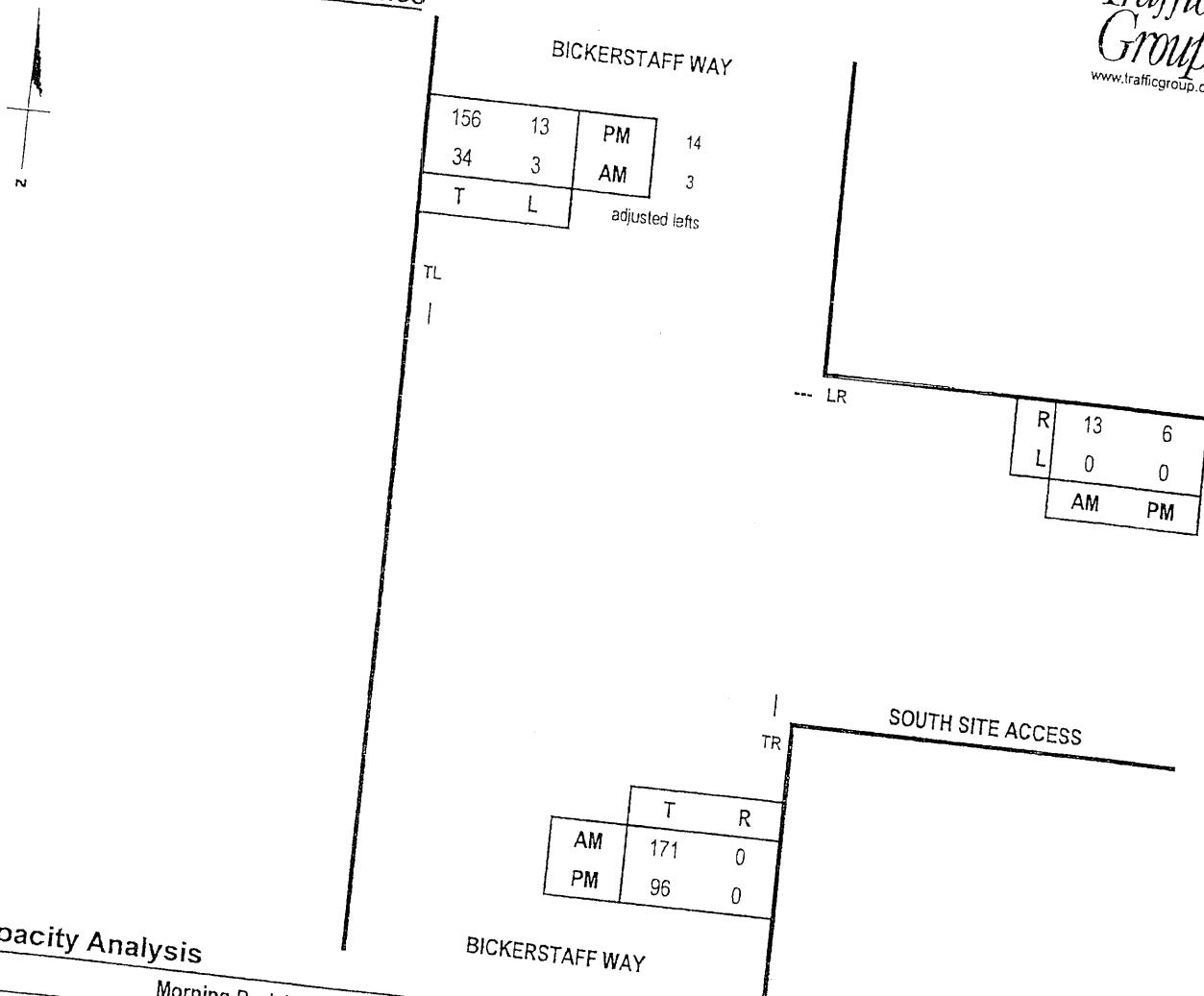
CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

Intersection of: Bickerstaff Way
and: South Site Access
Conditions: Total Traffic

Date of Count:
Day of Week:
Analyst: Shulin Li

Lane Use + Traffic Volumes



Capacity Analysis

Morning Peak Hour					
Dir	Thru Volumes			+ Opposing Lefts	
	VOL	x LUF	= Total	VOL	x LUF = Total
WB	13	1.00	13		13
NB	171	1.00	171	3	1.00 3
SB	37	1.00	37		174

CLV TOTAL= 187
Level of Service (LOS)= A
AM V/C = 0.12

Evening Peak Hour					
Dir	Thru Volumes			+ Opposing Lefts	
	VOL	x LUF	= Total	VOL	x LUF = Total
WB	6	1.00	6		6
NB	96	1.00	96	13	1.00 13
SB	169	1.00	169		169

CLV TOTAL= 175
Level of Service (LOS)= A
PM V/C = 0.11

CRITICAL LANE VOLUME (CLV) METHODOLOGY

for Montgomery County

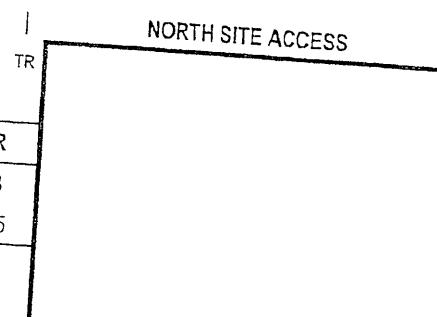
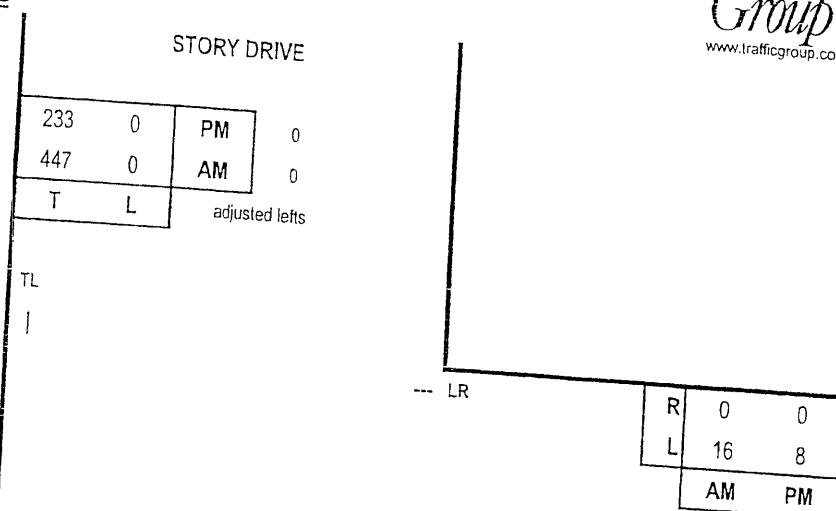
sli_04101118.xls-clv, 11/23/04

Intersection of: Story Drive
and: North Site Access
Conditions: Total Traffic

Date of Count:
Day of Week:
Analyst: Shulin Li



Lane Use + Traffic Volumes



STORY DRIVE

Capacity Analysis

Morning Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		AM CLV
	VOL	x LUF	= Total	VOL	x LUF	
WB	16	1.00	16			16
NB	103	1.00	103	0	1.00	0
SB	447	1.00	447			447
CLV TOTAL=				463		
Level of Service (LOS)=				A		
AM V/C = 0.29						

Scenario ID - TOTB

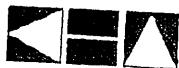
Evening Peak Hour						
Dir	Thru Volumes			+ Opposing Lefts		PM CLV
	VOL	x LUF	= Total	VOL	x LUF	
WB	8	1.00	8			8
NB	310	1.00	310	0	1.00	0
SB	233	1.00	233			310
CLV TOTAL=				318		
Level of Service (LOS)=				A		
PM V/C = 0.2						

APPENDIX C

Background Development

Information





Kimley-Horn
and Associates, Inc.

13755 Sunrise Valley Drive
Suite 450
Herndon, Virginia 20171

Date: Oct 26 2004

TEL 703.674.1300
FAX 703.674.1350

Fax Transmittal - Please note new address, phone and fax number above

To: Wes Grueter Fax No.: 1-410-931-6601
Firm/Location: The Traffic Group Job No.: _____
From: Ed Peperzak Job Name: _____
Original coming by mail: Yes No
If you have any problems, please call 703.674.1300 and ask for: ED
Total number of pages, including cover sheet: 9

Comments:

Wes:

Here's what I have that may help.
From Nov 7, 2001 memo -

(1) Cover page

(2) List of approved and unbuilt developments.
(The Washingtonian North parcel is not built. I'm not sure about the others.)

Directions of Approach

(3) Trip assignment of approved and unbuilt developments
Note - trips based on 50th Edition of Trip Generation

(4) Trip generation of Washingtonian Waterfront

(5) Graphic showing assignment of Wash. Waterfront trips.

From Apr 16, 2002 memo with revised quantity for Washingtonian Waterfront.
(I don't know what was finally approved)

(6) Cover page

(7) Trip generation

(8) Directions of Approach

This memo deals with a specific site driveway issue and did not include trip assignments on to surrounding roadways.

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Hope this is useful
Ed



Kimley-Horn
and Associates, Inc.

MEMORANDUM

To: Jeff Parana
The Peterson Companies

From: Edward Y. Papazian, PE
Robert B. Schiesel

Date: November 7, 2001

Subject: Washingtonian Waterfront
Traffic Impact Analysis

■
Suite A
9411 Lee Highway
Fairfax, Virginia
22031

INTRODUCTION

This memorandum presents the results of an updated traffic analysis for the proposed Washingtonian Waterfront mixed-use development in Gaithersburg, Maryland. The site is part of the Washingtonian Center development and is located on Washingtonian Boulevard south of its interchange with Sam Eig Highway. Figure 1 is a study area map.

This analysis is an update of the previous studies conducted for the Washingtonian Center and is based on current traffic counts in the area. This study was conducted in accordance with direction provided by the Public Works department of the City of Gaithersburg. This study analyses the impact of the proposed Washingtonian Waterfront development on four local intersections. It also addresses specific items requested by City of Gaithersburg staff.

AREA TRANSPORTATION SYSTEM

The following is a description of the key roadways that serve the Washingtonian Center area.

Existing Roadway System

The key roadways in the study area are Sam Eig Highway, Washingtonian Boulevard, Fields Road and Diamondback Drive. Figure 2 shows the roadway system and lane designations at area intersections.

latest Trip Generation report prepared by the Institute of Transportation Engineers (ITE)¹. The list of approved and unbuilt developments and the resulting number of trips are shown in Table 1.

Name	Land Use (Code)	Amount	AM Peak Hour			PM Peak Hour		
			In	Out	Two-Way	In	Out	Two-Way
Two Washingtonian Center	Office (710)	297,615	392	53	445	70	343	413
Washingtonian North	Office (710)	850,000 (three buildings ¹)	1,128	154	1,282	202	989	1,191
South Seven	Office (710)	300,000 (two buildings ²)	454	62	516	84	410	494

¹ - 330,000 SF, 275,000 SF and 245,000 SF
² - Two buildings each of 150,000 SF

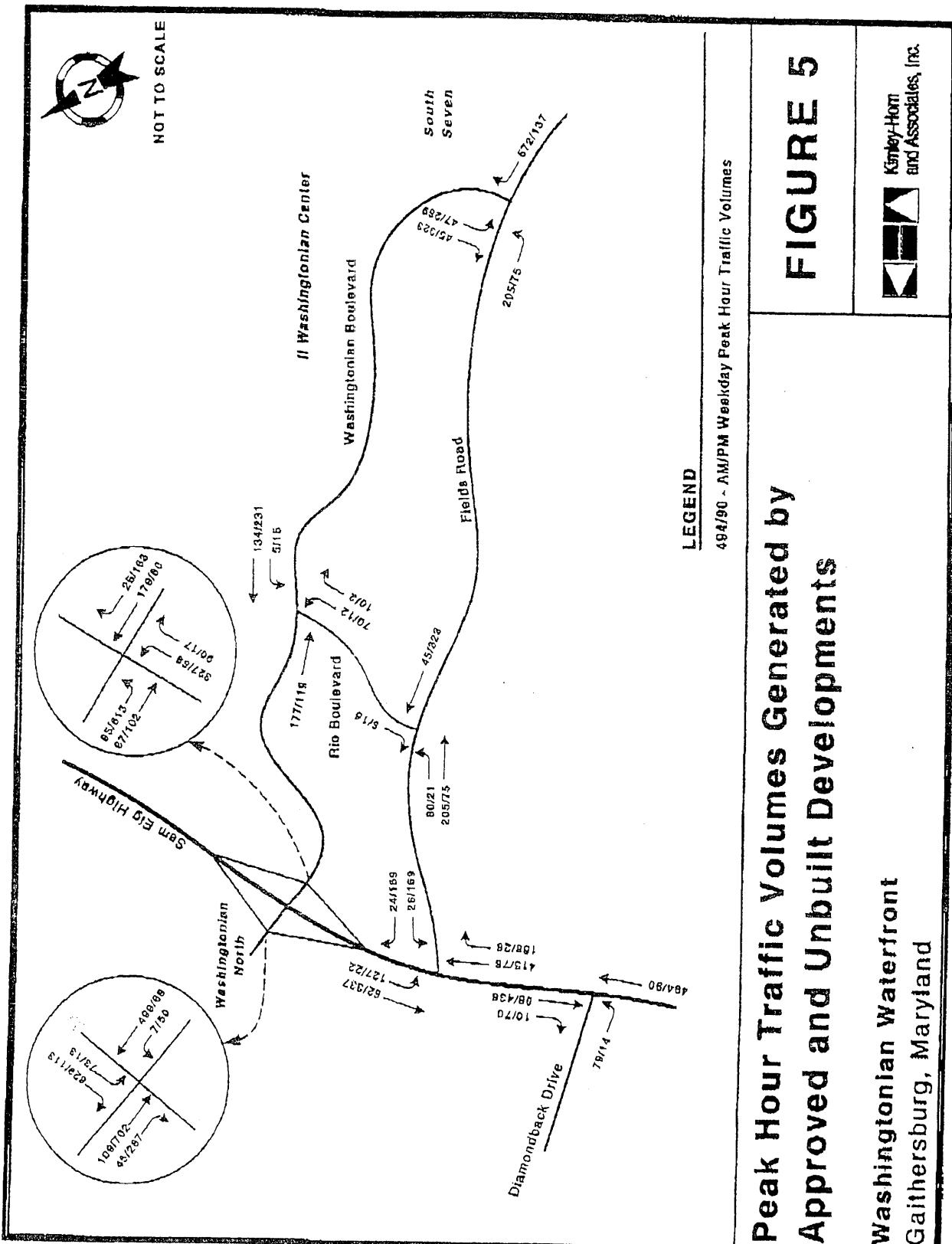
These volumes were distributed through the study area intersections based on the directions of approach of development traffic, shown in Table 2.

Table 2 - Directions of Approach of Development Traffic			
Direction From/To	Residential Development	Office Development	Retail Development
North on I-270	19%	26%	28%
East on I-370/Shady Grove Road	15%	12%	8%
South on I-270	37%	24%	24%
South and East on Fields Road	17%	9%	12%
South on Sam M. Sieg Highway	8%	25%	26%
West on Diamondback Drive	4%	4%	2%

These directions of approach are those contained in previous traffic studies conducted for the Washingtonian Center

The trip assignments for approved developments are shown on Figure 5. Background traffic for this study is comprised of the existing traffic volumes (Figure 4), and the traffic generated by approved and unbuilt or unoccupied parcels on the Washingtonian Center. These volumes added together are displayed on Figure 6.

¹ Trip Generation, Institute of Transportation Engineers, Sixth Edition, 1997



Site Generated Traffic Volumes

Peak hour traffic volumes generated by the proposed Washingtonian Waterfront mixed-use development were calculated based upon the rates contained in the latest ITE Trip Generation report. The resulting trip generation figures are shown in Table 3.

Land Use and Quantity	Land Use Code	AM Peak Hour			PM Peak Hour		
		In	Out	Two-Way	In	Out	Two-Way
48,000 SF Specialty Retail Stores	814	16	15	31	53	71	124
18,000 SF High-Quality Restaurant	831	10	5	15	90	45	135
210-room Hotel	312	72	50	122	78	52	130
240 Apartments	222	18	54	72	54	34	88
Total		116	124	240	275	202	477

These trips were assigned to the area intersections based on the directions of approach of development traffic contained in Table 2. Figure 7 shows the assignment of trips generated by the proposed Washingtonian Waterfront.

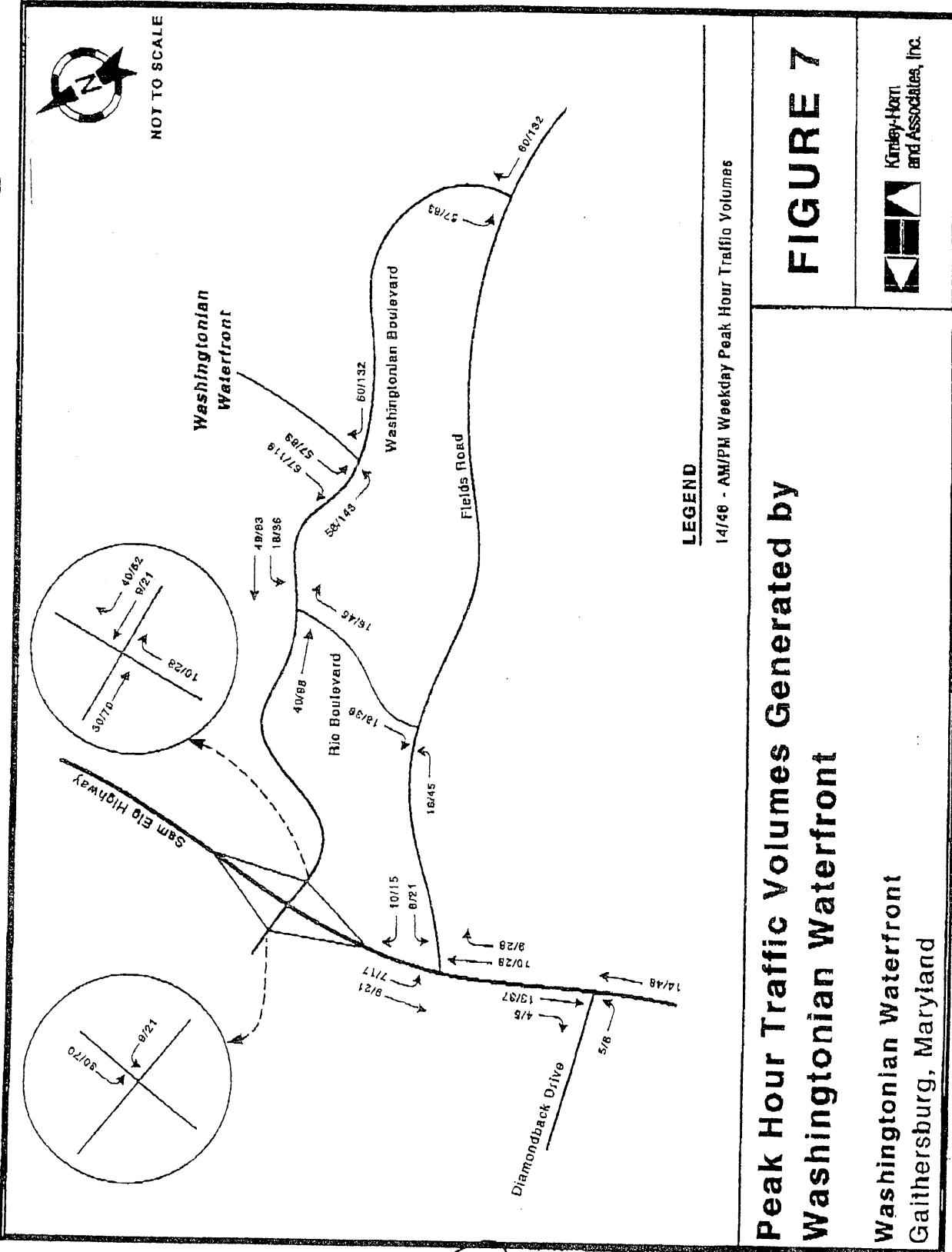
Total Future Traffic Volumes

Total future traffic volumes were calculated by adding the traffic generated by the proposed Washingtonian Waterfront (Figure 7), to background traffic volumes (Figure 6). The resulting total future traffic volumes are shown on Figure 8.

ASSESSMENT OF TRAFFIC CONDITIONS

Several different assessments of traffic conditions were conducted. They include intersection capacity analyses, traffic signal operations at the junction of the signalized ramp connections from Sam Eig Highway at Washingtonian Boulevard, weaving along Sam Eig Highway, adequacy of stacking distances along Washingtonian Boulevard into the Waterfront parcel, and adequacy of roundabout at Washingtonian Boulevard and Rio Boulevard.

(4)



5



Kimley-Horn
and Associates, Inc.

MEMORANDUM

To: Jeff Parana
The Peterson Companies

From: Edward Y. Papazian, P.E.

Date: April 16, 2003

Subject: Washingtonian Waterfront
Analysis of the Intersection of Washingtonian Boulevard and the Main
Driveway to Waterfront

Suite 450
13755 Sunrise Valley Drive
Herndon, Virginia
20171

INTRODUCTION

This memorandum presents an analysis of the intersection of Washingtonian Boulevard and the main driveway to the Washingtonian Waterfront development, and specifically the adequacy of the length of the southbound left turn lane along Washingtonian Boulevard into the Washingtonian Waterfront Parcel. The proposed development program for the Washingtonian Waterfront parcel consists of the following.

- 47,180 square feet of specialty retail stores
- 18,100 square feet of quality restaurants
- 210 room hotel
- 87,815 square feet of office

DESCRIPTION OF PREVIOUS STUDY

A traffic study dated November 7, 2001 contained detailed analyses of the adequacy of the area roadway system for a development program that was essentially the same as the current program, with the exception that the previous program included 240 apartment residential units in place of the office building. The November 7, 2001 study was based on existing traffic counts conducted in the fall of 2001 and included buildout of the Washingtonian Center. The study concluded that the area roadway system will accommodate the proposed development with acceptable levels of traffic service and demonstrated that the length of the left turn lane along Washingtonian Boulevard into the main driveway serving the Washingtonian Waterfront was more than sufficient to accommodate the peak hour left turn demand. Overall analysis of the intersection of

(6)

Washingtonian Boulevard and the driveway to the Waterfront was not part of this study. The November 2001 study included an analysis of an alternate development plan for the Washingtonian Waterfront involving substituting a 125,000 square foot office building in place of the 240 residential units. The study concluded that traffic generated by the alternate plan with the office building would be accommodated on the area roadway system. The intersection of Washingtonian Boulevard and the driveway to the Waterfront was not evaluated with this alternate plan.

The following section describes the analysis of the intersection with the current proposed plan for the Washingtonian Waterfront parcel.

INTERSECTION ANALYSIS

Peak hour traffic volumes generated by the proposed Washingtonian Waterfront were calculated based upon trip generation rates contained in the latest ITE Trip Generation report. The resulting trip generation figures are shown in Table 1.

Table 1 – Traffic Generated by Proposed Washingtonian Waterfront

Land Use and Quantity	Land Use Code	AM Peak Hour			PM Peak Hour		
		In	Out	Two-way	In	Out	Two-way
47,180 SF Specialty Retail Stores	814	16	15	31	52	70	122
18,100 SF High-Quality Restaurant	831	10	5	15	91	45	136
210 Room Hotel	312	72	50	122	78	52	130
87,815 SF Office	710	148	20	168	30	148	178
Total	---	246	90	336	251	315	566

These trips were assigned to Washingtonian Boulevard based on the same distributions contained in the November 2001 study. These distributions are shown in Table 2.

Vehicle access along Washingtonian Boulevard serving the Waterfront parcel consists of a main driveway with full movements permitted and a right turn in and right turn out driveway located approximately 200 feet to the north. The focus of this analysis is the main driveway.

Table 2 — Directions of Approach of Development Traffic

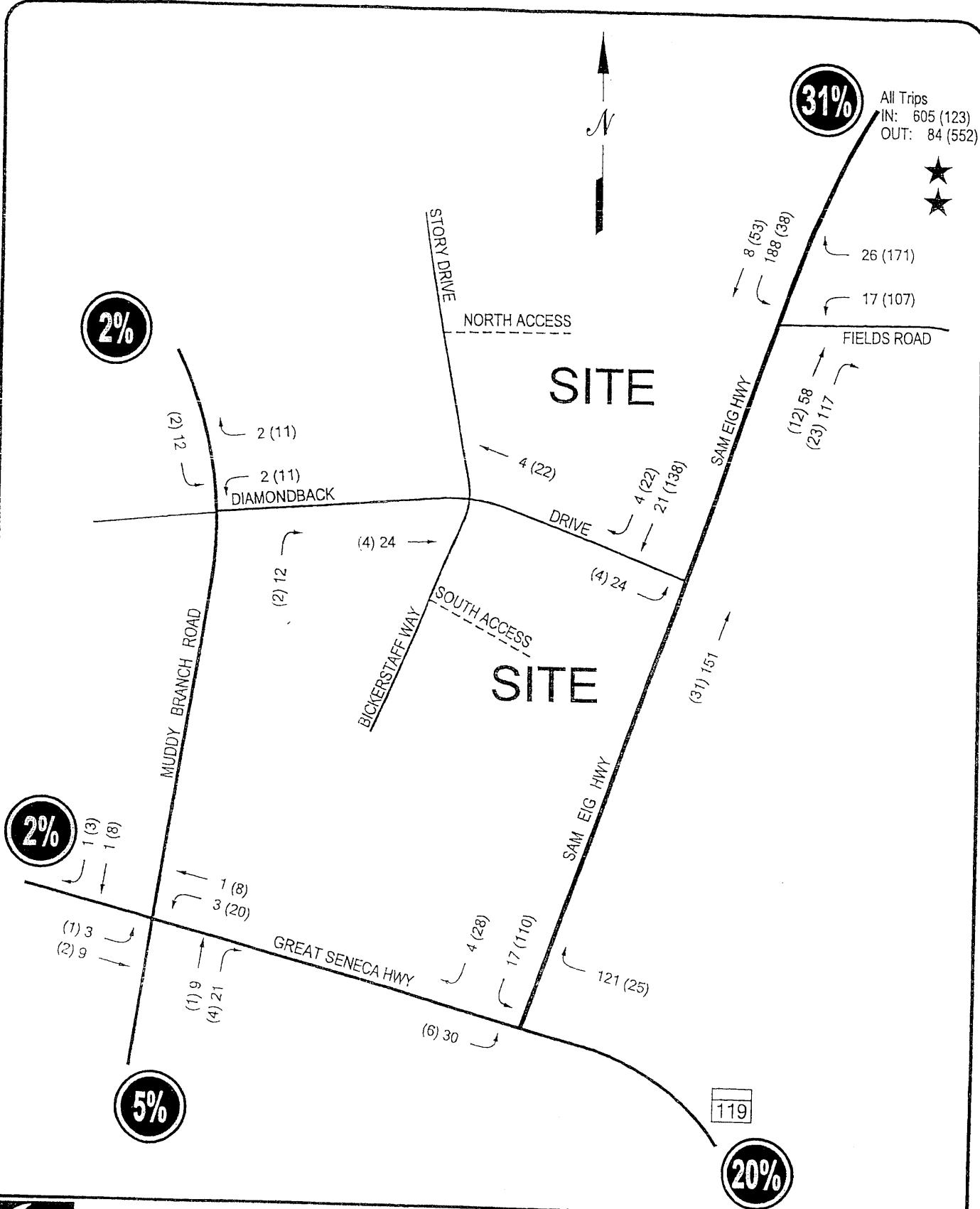
Directions From/To	Office/Hotel	Retail
North on I-270	26%	28%
East on I-370/Shady Grove Road	12%	8%
South on I-270	24%	24%
South and East on Fields Road	9%	12%
South on Sam Ely Highway	25%	26%
West on Diamondback Drive	4%	12%

Through traffic along Washingtonian Boulevard was taken from the August 21, 1998 traffic study for the Washingtonian North parcel. The estimated through traffic in the 1998 study was based on existing traffic counts conducted during the course of the development of the Washingtonian Center plus the forecast traffic resulting from the remaining buildout.

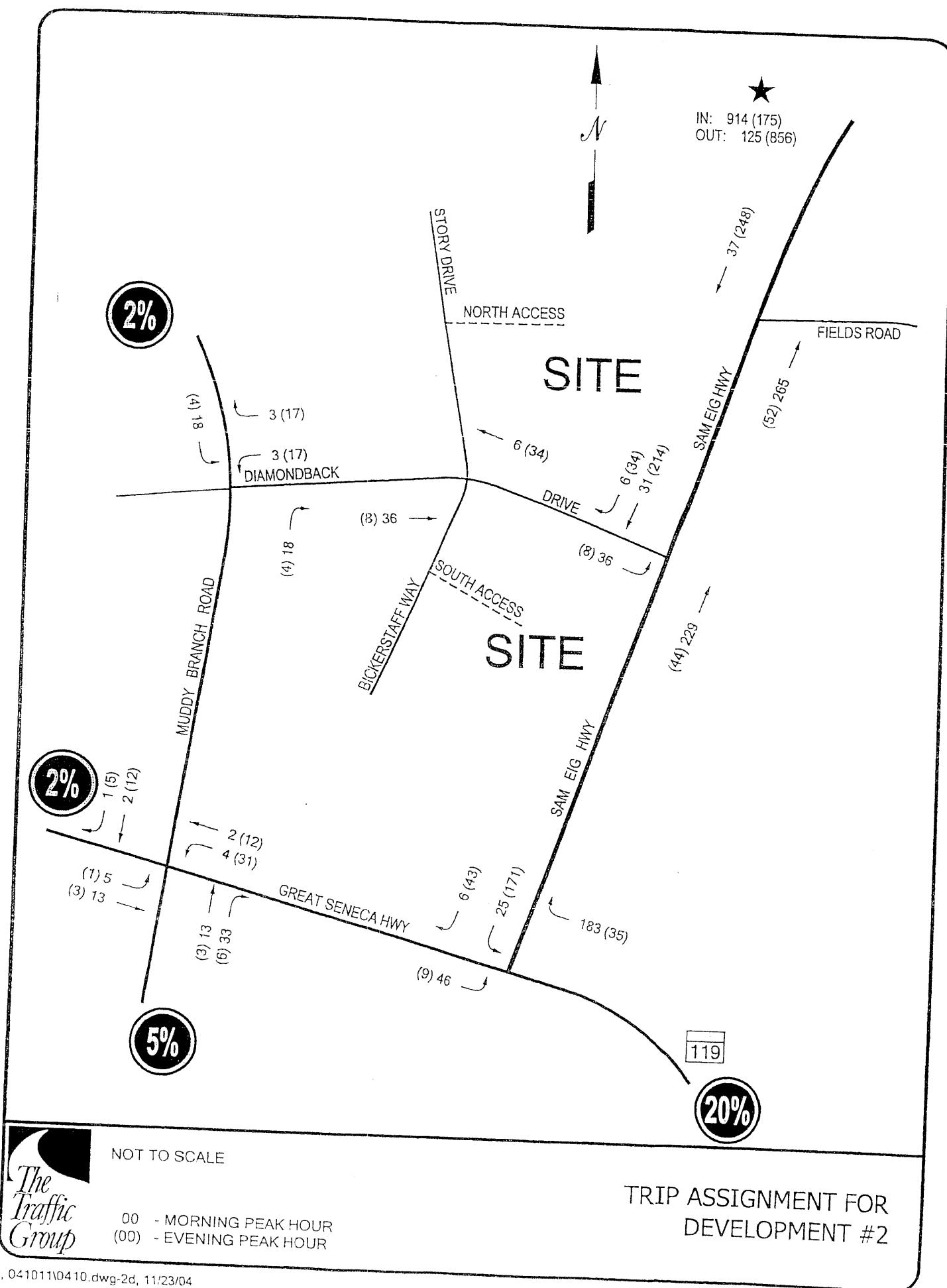
Figure 1 shows the estimated through traffic along Washingtonian Boulevard and the assignment of Washingtonian Waterfront entering and exiting traffic at the driveways serving the Waterfront parcel.

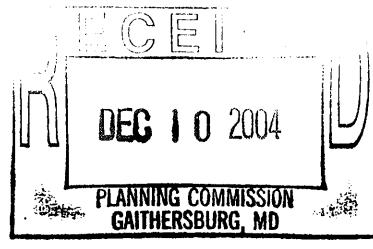
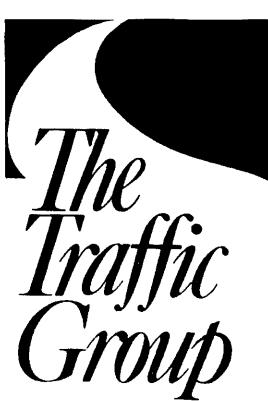
The trips generated by the Waterfront parcel were assigned to the driveways based on the assumption that 90 percent of the right turn in and 50 percent of the right turn out traffic and all of the left turn in and left turn out traffic would use the main driveway. The remaining 10 percent of the right turn in and 50 percent of the right turn out traffic would use the right turn in and right turn out driveway.

The lane designations at the intersection of the main driveway are shown on Figure 2. The intersection will be stop sign controlled with the stop sign controlling the exiting movement from the driveway.



TRIP ASSIGNMENT FOR
DEVELOPMENTS #1 & 3





December 8, 2004

North Gaithersburg Investments, LLC
2401 Research Boulevard
Suite 202
Rockville, Maryland 20850

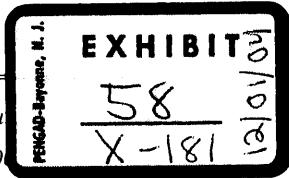
RE: Crown Farm
Accident Data
Gaithersburg, Maryland
Our Job No: 2004-1011

Dear North Gaithersburg Investment, LLC:

As discussed, The Traffic Group, Inc. has had an opportunity to obtain accident data from the Maryland State Highway Administration Accident Data Files to determine the number of accidents that have occurred at the Sam Eig Highway and Diamondback Drive intersection in Montgomery County. This information was obtained for the years 2001 through 2003, which is the most current information available from the Maryland State Highway Administration at this time.

Attached to this letter is Exhibit 1 which shows a breakdown of the accidents that have been recorded at the Sam Eig Highway and Diamondback Drive intersection for the years 2001 through 2003. A review of Exhibit 1 indicates that over this three year period, a total of 21 accidents have occurred at the subject intersection. This would equate to an average of seven accidents per year. A roadway with the amount of traffic that Sam Eig Highway serves, seven accidents per year would equate to an Accident Rate of 0.89 per MVE (million vehicles entering) which is low and would fall below the State Average for a Signalized Intersection.

Also attached is Exhibit 2 which is a Collision Diagram which illustrates the types of accidents and the accident patterns based on the information contained on Exhibit 1 for the past three years. Several of these accidents were rear-end accidents which



are common and expected to occur at a signalized intersection. A full summary of each accident for each year is also attached.

In summary, based on the accident information provided, it is our opinion that the intersection of Sam Eig Highway and Diamondback Drive has a low number of accidents for a road that carries the amount of traffic that Sam Eig Highway carries on a daily basis and is below the State Average. Therefore, it is our opinion that the signalized intersection of Sam Eig Highway and Diamondback Drive is not experiencing any safety concerns.

If you have any questions concerning the attached information, please do not hesitate to contact me.

Sincerely,



Glenn E. Cook
Senior Project Manager

GEC:clg

(F:\2004\2004-1011\wp\Sears.doc)

	2001	2002	2003	3 Year Total	1 Year Average
<u>Roadway Intersections</u>					

*Sam Eig Highway (0.41 mp)
at Diamondback Drive (0.35 mp)

4

6

11

21

7.0

*intersection only

<u>Accident Frequency</u>	4	6	11	21	7.0
<u>Accident Location:</u>					
Intersection Related	4	6	11	21	7.0
Non-intersection	0	0	0	0	0.0
<u>Accident Severity:</u>					
Not Injured	2	2	3	7	2.3
Possible Injury	2	1	2	5	1.7
Injured	0	1	4	5	1.7
Disabled	0	2	2	4	1.3
Fatal	0	0	0	0	0.0
<u>Injuries Involved:</u>					
Occupants Killed	0	0	0	0	0.0
Pedestrians Killed	0	0	0	0	0.0
Occupants Injured	3	5	15	23	7.7
Pedestrians Injured	0	0	0	0	0.0
<u>Vehicles Involved:</u>					
Single Vehicle	1	0	0	1	0.3
Two Vehicles	3	6	8	17	5.7
Three or More	0	0	3	3	1.0
<u>Collision Type:</u>					
Unknown	0	0	0	0	0.0
Other	0	0	1	1	0.3
Hit and Run	0	0	0	0	0.0
Rear-end	0	0	4	4	1.3
Sideswipe	0	0	1	1	0.3
Angle	1	4	5	10	3.3
Opposite Direction	1	2	0	3	1.0
Single Vehicle:	2	0	0	2	0.7
Pedestrian	0	0	0	0	
Bicycle	0	0	0	0	
Animal	0	0	0	0	
Fixed Object	1	0	0	1	
Other	1	0	0	1	



EXHIBIT 1

3 YEAR CRASH REVIEW SUMMARY (2001 - 2003) SAM EIG HIGHWAY & DIAMONDBACK DRIVE page 1 of 2

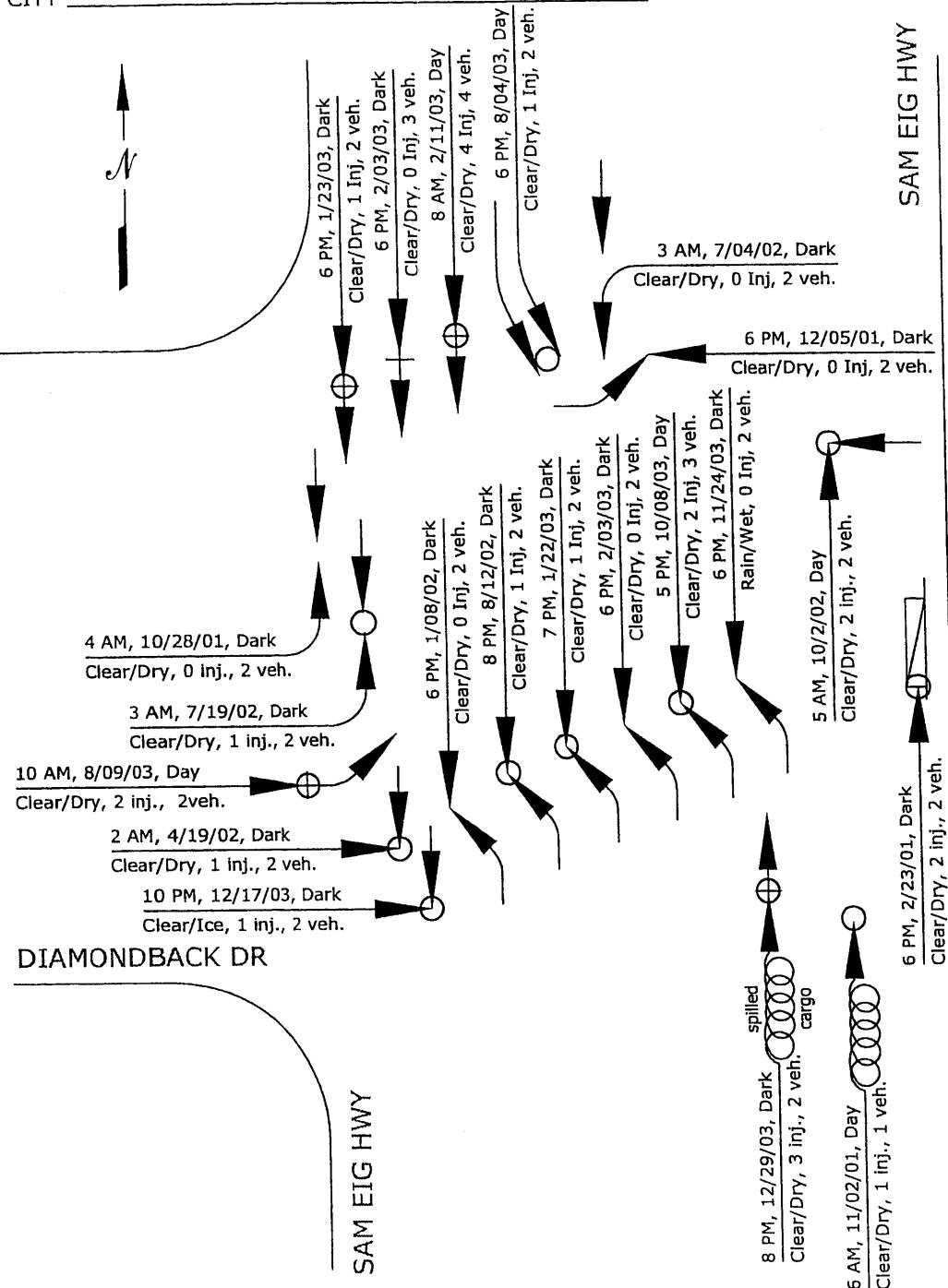
	2001	2002	2003	3 Year Total	1 Year Average
<u>Weather:</u>					
Clear/Cloudy	4	6	10	20	6.7
Foggy	0	0	0	0	0.0
Raining	0	0	1	1	0.3
Snow/Sleet	0	0	0	0	0.0
Other	0	0	0	0	0.0
<u>Light:</u>					
Daylight	1	1	4	6	2.0
Dawn/Dusk	0	0	0	0	0.0
Dark (lights on/off)	3	5	7	15	5.0
Other	0	0	0	0	0.0
<u>Surface Condition:</u>					
Dry	4	6	9	19	6.3
Wet	0	0	1	1	0.3
Snow/Sleet	0	0	0	0	0.0
Ice	0	0	1	1	0.3
Other	0	0	0	0	0.0
<u>Time of Day:</u>					
12 MN - 5 AM	1	4	0	5	1.7
AM 6 AM - 9 AM	1	0	1	2	0.7
MD 10 AM - 3 PM	0	0	1	1	0.3
PM 4 PM - 7 PM	2	1	7	10	3.3
8 PM - 11 PM	0	1	2	3	1.0
Unknown	0	0	0	0	0.0
<u>Day of Week:</u>					
Monday	0	1	5	6	2.0
Tuesday	0	1	1	2	0.7
Wednesday	1	1	3	5	1.7
Thursday	0	1	1	2	0.7
Friday	2	2	0	4	1.3
Saturday	0	0	1	1	0.3
Sunday	1	0	0	1	0.3
<u>Time of Year (by quarter):</u>					
1st January - March	1	1	5	7	2.3
2nd April - June	0	1	0	1	0.3
3rd July - September	0	3	2	5	1.7
4th October - December	3	1	4	8	2.7
<u>Number of Accidents</u>	4	6	11	21	7.0



EXHIBIT 1
3 YEAR CRASH REVIEW SUMMARY (2001 - 2003)
SAM EIG HIGHWAY & DIAMONDBACK DRIVE
page 2 of 2

COLLISION DIAGRAM

INTERSECTION SAM EIG HIGHWAY AND DIAMONDBACK DRIVE
 PERIOD 3 YEARS FROM 1 JAN. 2001 TO 31 DEC. 2003
 CITY MONTGOMERY COUNTY PREPARED BY Mgraham



ACCIDENTS:

2001 - 4
2002 - 6
2003 - 11

EXHIBIT 2

COLLISION DIAGRAM
YEARS 2001 - 2003 (3 YEARS)
SAM EIG HWY & DIAMONDBACK DR



LEGEND:

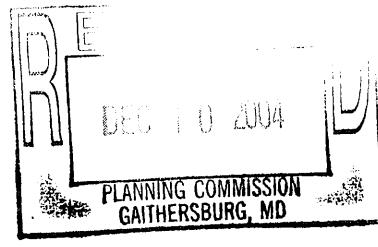
- MOVING VEHICLE
- BACKING VEHICLE
- ←→ NON-INVOLVED VEHICLE
- PEDESTRIAN
- ↔ PARKED VEHICLE
- FIXED OBJECT
- FATAL ACCIDENT
- INJURY ACCIDENT

Data Entry

ID#	Report #	LM	Date	DA	TD	IR	AS	OK	PK	OI	PI	W	L	S	#V	V1	V2	CT	FH	SH	FO	D1	M2	M1	D2	A	PC	SC	CD1	CD2	MOY					
1	09038713	0.40	2/11/2003	3	8	3	4	0	0	4	0	1	1	2	4	2	2	3	1	1	99	0	2	1	6	S	3	7	0	1						
2	08483748	0.41	1/22/2003	4	19	2	2	0	0	1	0	1	3	2	2	21	2	2	2	3	1	99	0	1	2	12	1	2	3	6	S	1	7	0	1	
3	08483743	0.41	1/23/2003	5	18	3	3	0	0	1	0	1	3	2	2	20	2	2	2	3	1	99	0	2	1	2	12	1	2	3	6	S	1	11	0	1
4	09724909	0.41	2/3/2003	2	18	2	1	0	0	0	0	0	1	3	2	2	2	2	2	2	1	9	5	1	2	12	1	2	12	1	N	L	7	0	1	
5	09724909	0.41	10/8/2003	4	17	2	3	0	0	2	0	1	1	2	3	2	2	2	2	1	99	0	1	2	12	1	2	12	1	S	2	11	0	1		
6	10060531	0.41	11/24/2003	2	18	2	1	0	0	0	0	0	3	4	1	2	2	2	2	2	1	99	0	1	2	12	1	2	12	1	S	2	7	0	1	
7	10060599	0.41	12/17/2003	4	22	2	3	0	0	1	0	1	3	4	2	2	2	2	2	2	1	11	1	0	0	2	3	3	4	3	S	2	7	0	1	
8	10059982	0.41	12/29/2003	2	20	2	4	0	0	3	0	1	3	2	2	2	2	2	2	2	20	88	2	12	0	1	1	6	N	2	7	0	1			
9	09769858	0.45	8/4/2003	2	18	3	3	0	0	1	0	1	1	2	2	2	2	2	2	10	1	99	4	2	2	13	E	1	15	0	1					
10	08483745	0.47	2/3/2003	2	18	3	1	0	0	0	0	0	1	3	2	2	2	2	2	1	0	0	0	2	2	3	6	S	2	7	0	1				
11	08475957	0.35	8/9/2003	7	10	3	2	0	0	2	0	1	1	2	2	2	2	2	2	1	0	0	3	3	1	4	E	L	7	0	1					



The
Traffic
Group



December 8, 2004

North Gaithersburg Investment, LLC
2401 Research Boulevard
Suite 202
Rockville, MD 20850

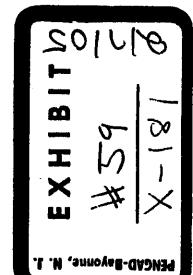
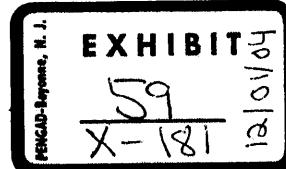
RE: Crown Farm
Gaithersburg, Maryland;
Possible grade separated interchange at
Sam Eig Highway and Diamondback Drive
Our Job No: 2004-1011

Dear North Gaithersburg Investment, LLC:

The Traffic Group, Inc. has recently completed a Traffic Impact Analysis for the proposed development of the Crown Farm which is located along the west side of Sam Eig Highway just south of its intersection with Fields Road. As part of this Traffic Impact Study, it was requested by the City of Gaithersburg, that the intersection of Sam Eig Highway and Diamondback Drive be evaluated.

The Traffic Impact Analysis that was conducted dated December 8, 2004, indicated that the intersection of Sam Eig Highway and Diamondback Drive was projected to operate at a Level of Service "C" during the morning peak period and a Level of Service "D" during the evening peak period based on the build-out of the developments included in the analysis. This Traffic Impact Analysis took into consideration the development of the Crown Farm, which is planned to consist of 80 townhouse units and two other nearby developments. As part of the background traffic conditions, the traffic projected to be generated by the Washingtonian Center (north and south), which is planned to consist of 1.2 million square feet of office space, and the Rio Waterfront Property, which is an additional 106,000 square feet of commercial space were included in the analysis. When assuming the development of these properties and the Crown Farm, the intersection of Sam Eig Highway and Diamondback Drive is still projected to operate at satisfactory levels of

The Traffic Group, Inc. 9900 Franklin Square Drive
Phone: 410-931-6600 Fax: 410-931-6601 Toll Free:



Attachment "K"

service with over 1.3 million square feet of commercial space yet to be developed in this area.

The 1990 Shady Grove Study Area Master Plan suggests a possible grade separated interchange at this location. However, this design was schematic and "illustrative only". The Master Plan further states "...that equivalent at-grade solutions may work and be more appropriate." It is clear from our analysis that the traffic projections that were developed fifteen years ago have not materialized and, based on our analysis and projections, a grade separated interchange is not necessary at this location.

Therefore, based on current information, the at-grade intersection is more than capable of supporting the development planned in this area.

If you have any further questions, please do not hesitate to contact me.

Sincerely,



Glenn E. Cook
Senior Project Manager

GEC:clg

(F:\2004\2004-1011\wp\North Gaithersburg.DOC)



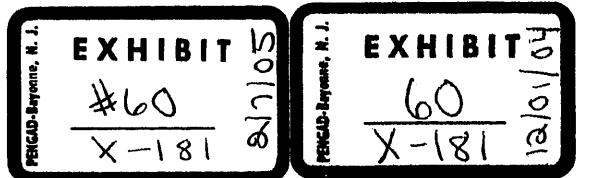
9 December 2004

Phoenix Noise & Vibration, LLC
930 North East Street, Suite 4
Frederick, Maryland 21701
301.846.4227 (phone)
301.846.4355 (fax)
www.phoenixnv.com

Traffic Noise Analysis Crown Farm Property Montgomery County, Maryland

Report #041209

For: North Gaithersburg Investment, LLC
By: Scott Harvey, P.E.



Executive Summary

Phoenix Noise & Vibration has conducted a noise impact analysis on the proposed townhouse development at the Crown Farm Property in Montgomery County, Maryland. This analysis included on-site noise measurements, traffic noise modeling, future traffic noise level analysis, determination of proposed townhome barrier effect and recommendations.

Results indicate that outdoor noise levels in rear yards, most open space, and tot lots will meet Montgomery County Guidelines for residential use. Some buildings will be impacted by traffic noise, however with proper construction techniques, recommended interior noise level requirements can be met.

Noise Regulations

Traffic noise impact for proposed residential developments in Montgomery County is governed by "Staff Guidelines for the Consideration of Transportation Noise Impacts In Land Use Planning and Development", June 1983. Specifically for residential developments, the guidelines are presented in Table 2-1 on page 8, as reprinted here:

Table 2-1
Maximum Levels for Exterior Noise ant the Building Line¹
For Noise Sensitive Land Uses

Guideline Value	Area of Application
Ldn = 55 dBA	This guideline is suggested as an appropriate goal in permanent rural areas of the County where residential zoning is for five or more acres per dwelling unit and background levels are low enough to allow maintenance of a 55 dBA Level. This guideline is consistent with Federal, State, and County goals for residential areas.
Ldn = 60 dBA	This is the basic residential noise guideline which will be applied in most areas of the County where suburban densities predominate. Maintenance of this level will protect health and substantially prevent activity interference both indoors and outdoors. Noise attenuation measures will be recommended to allow attainment of this level.
Ldn = 65 dBA	This guideline will generally be applied in the urban ring, freeway, and major highway corridor areas, where ambient levels are such that application of a stricter guideline would be infeasible or inequitable. Significant activity interference will occur outdoors and indoors if windows are partially opened, but available evidence indicates hearing is adequately protected. Noise attenuation measures will be strongly recommended to attain this level.

¹ Building line as used here refers to habitable structures only. It does not include garages, sheds, or recreational accessory buildings.

Given the close proximity to Sam Eig Highway for the proposed site and references made to highways and freeways in the guidelines, the 65 dBA Ldn guideline value is chosen as most appropriate for the Crown Farm Development.

Noise Measurements

On November 18 and 19, 2004, Phoenix NV personnel conducted an on-site measurement survey to determine the current levels of traffic noise on the site. This involved continuous noise level measurement and monitoring for one continuous 24-hour period. During that period, 30 minute Leq values were recorded. Leq is the average noise level over some designated time period; in this case that time period is 30 minutes. These measured levels were in turn used to calculate the Day-Night Average Noise Level or Ldn, which is consistent with the County guidelines.

Measurements were made in two locations, designated as "A" and "F", the results of which are provided in the following table:

Noise Measurement Location	Day Night Avg. Noise Level, Ldn
A	63.3 dBA
F	67.3 dBA

The exact locations of the measurement points are shown on the enclosed site plan. These points were determined to be worst-case locations (those areas on the site where noise levels were highest) based upon topography and proposed location of the townhomes. Both points were in roughly the same vertical plane as the roadway and in clear line of site with the roadway. Notice that Point A experiences lower noise levels than Point F. This is due to the presence of a hill to the north of the site which shields Point A from portions of the traffic noise.

Computer Modeling

In order to determine the future noise impact on the property as well as the effects of proposed buildings on the rear yards, the Crown Farm site was computer modeled using the Federal Highway Administration's Traffic Noise Model (TNM). This is a three dimensional model capable of determining traffic noise levels based upon roadway characteristics such as width, grade, and speed, traffic volumes, site topography and ground cover, and distances to the roadway. Information from the site survey, the site plans, as well as WSSC database for roadway's layout and topography, was used to create the computer model.

Calibration Run

A calibration model was first created which produced noise level results that match the actual on site measurements by Phoenix NV. This is referred to as the calibration run. The site was modeled as is with current roadway layout and significant topographical features. The traffic control signal was also modeled in this run to simulate its effect on traffic flow. The results of the calibration run are presented in the following table along with the measured noise levels for comparison.

Noise Measurement Location	Day Night Avg. Noise Level, Ldn	
	Calibration Run Results	On site Measured Results
A	65.1	63.3 dBA
F	67.7	67.3 dBA

While the results at Point A differ by almost 2 dBA, those at Point F are within an acceptable margin and the overall results considered acceptable. Accepting the higher results for Point A simply errs toward a conservative result.

Future Run with and without townhomes

After the creation of the calibration run, a Future Run was created to determine the effects of additional traffic on the Sam Eig Highway and the effects of site grading, roadways and townhomes.

This future run was based upon traffic volume increase factor of 1.32 provided by The Traffic Group. This increase factor is based upon an annual increase of 1.5% over the next 21 years. This increase was applied to current average daily traffic volumes of 36, 226 vehicles per day, as provided by Maryland Department of Transportation, State Highway Administration, in order to determine the future year 2025 noise levels.

Given the orientation of the buildings, the rear yards and outdoor areas will be shielded by the buildings. To determine this shielding effect, the townhomes were modeled as 25 foot high barriers. The model calculates the future noise levels both with and without townhomes in place. Noise levels were calculated at 37 different locations around the site, each designated by lot number, open space area, or a site measurement location.

A graphical representation of the model along with the input and output data for both model runs is presented in the appendix. To see the output at the particular point, see page with header "Results: Sound Levels" for each model run. Note that the relevant sound levels are listed in the "Calculated" column under both "No Barrier" and "With Barrier" headings.

The future modeling results in absence of the townhomes were used to delineate the "unmitigated" noise contours. These contours were determined for both the ground level and upper level conditions and are shown on the enclosed site plan.

Due to the complex nature of delineating noise contours after the presence of buildings on the site, post construction or "mitigated contours" have not been determined. This concept of "mitigated noise contours" is not generally accepted in the noise control industry. However, 37 points on the site have been modeled in strategic locations in order to show the overall effect of the townhomes on the site. These are mostly located in rear yard areas which can be seen in the graphical representation of the model in the appendix. Some locations are in the open space, designated as "OS" and in the tot lot. By comparing the noise level results presented in the table with the layout, one can clearly deduce what the expected noise levels on the site will be after construction of the townhomes.

Results and Recommendations

By reviewing the results, one can clearly see that the proposed townhomes will be impacted by noise levels as high as 68 to 69 dBA Ldn; however rear yard areas will be at or below 62 dBA. Many rear yards will be below 60 dBA. These conditions meet Montgomery County Guidelines for outdoor areas in residential developments.

Regarding impact upon the buildings, a house of standard construction in today's market will reduce noise levels as high as 65 dBA to an interior noise level of 45 dBA without modification. When exterior noise levels rise above 65 dBA it is uncertain whether or not required interior noise levels will be met. In this instance, the recommended noise level is 45 dBA Ldn.

To insure achievement of required interior noise levels, Sound Transmission Class ratings (STC) of building elements should be specified. For rooms with moderate window sizes (20% of exterior surface area or less) the following STC ratings will maintain 45 dBA interior levels for levels up to 68 dBA:

Building Element	STC rating
Walls	39 STC
Windows	28 STC
Doors	28 STC

An exterior wall constructed of 2x4 wood studs, a minimum exterior layer of either $\frac{1}{2}$ -inch exterior grade drywall, OSB, or plywood with an interior layer of drywall and $3\frac{1}{2}$ inches of fiberglass batt in the cavity will achieve a rating of 39 STC.

An exterior wall constructed of 2x4 wood studs, 4-inch brick façade and an interior layer of drywall and $3\frac{1}{2}$ inches of fiberglass batt in the cavity will achieve a rating of 56 STC.

Properly constructed, a $\frac{1}{2}$ inch insulated window composed of two $\frac{1}{8}$ -inch panes separated by a $\frac{1}{4}$ -inch air space is capable of achieving a rating of 28 STC. It is strongly recommended that test results from a certified third party laboratory be obtained from prospective suppliers before purchase.

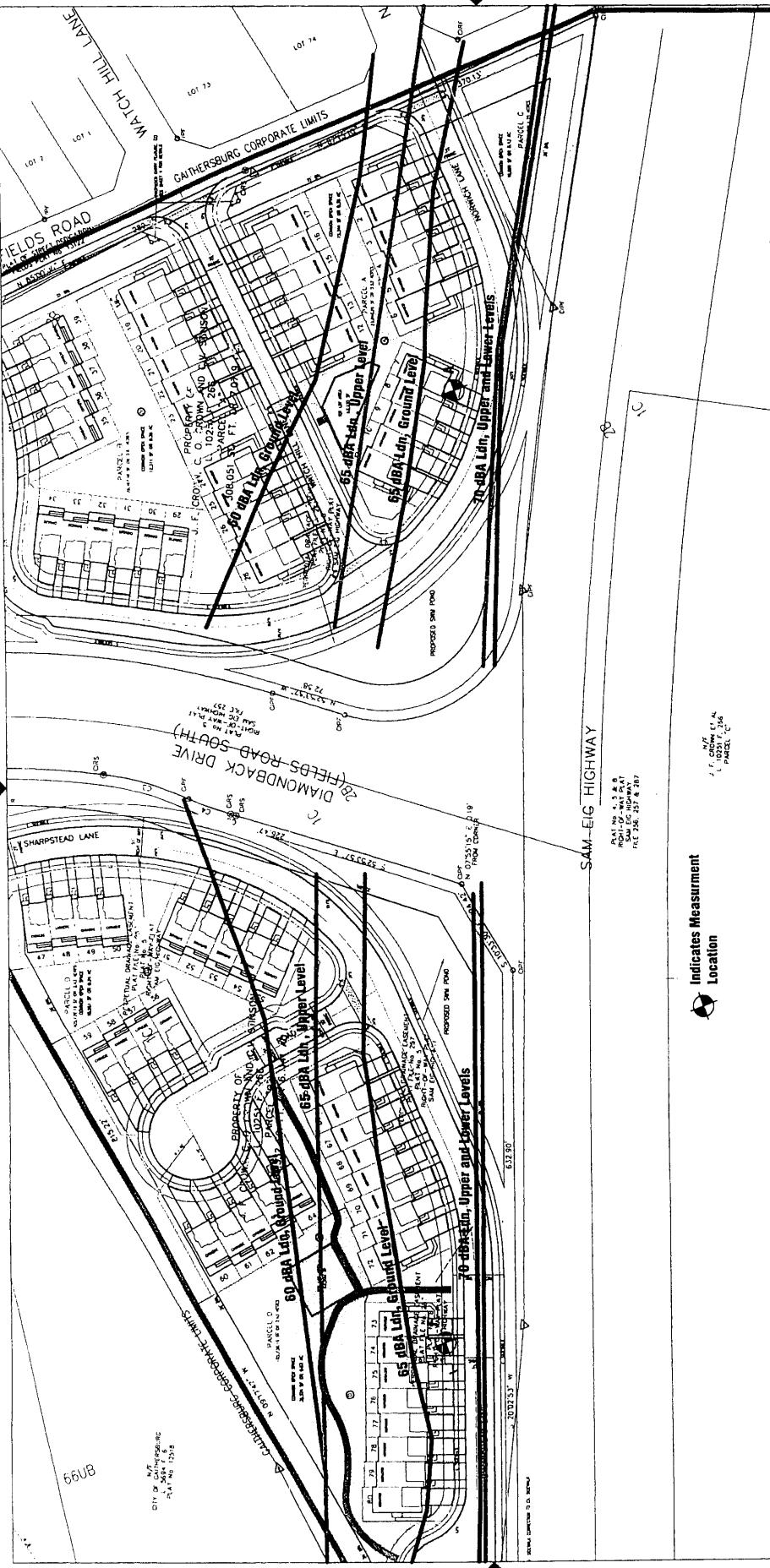
Properly constructed, a standard insulated metal door with a magnetic weather strip is capable of achieving a rating of 28 STC. It is strongly recommended that test results from a certified third party laboratory be obtained from prospective suppliers before purchase.

For units impacted by levels higher than 68 dBA or units with window areas exceeding 20%, it is strongly recommended that a building shell analysis be carried out for the specific units in order to determine the required STC ratings for each building element. If the proposed townhome units have rooms that have windows comprising more than 20% of the exterior façade, a building shell analysis is recommended for the front row of houses along Sam Eig Highway. This will insure achievement of the recommended 45 dBA Ldn interior noise level.

Conclusions

The proposed rear yards, open space, and tot lot areas will meet Montgomery County Guidelines for traffic noise impact. Some townhome units will be impacted by levels as high as 69 dBA. A building shell analysis may be required if the amount of window and door surface areas exceeds 20%.

Appendix



Crown Farm Traffic Noise Impact

Phoenix

Noise & Vibration, LLC
930 North East Street, Suite 4
Frederick, Maryland 21701
301-846-4227

Unmitigated Year 2025 Noise Contours

SIZE	FSCM NO.	DNC NO.	REV
SCALE	WRS	8 December 2004	SHEET



Indicates Measurement Location

Note: All noise contours are for the pre-construction, year 2025 condition.
Upon construction of proposed buildings, noise levels will be reduced significantly, due to barrier effect of buildings. In the post construction condition, all rear yard noise levels will be below 62 dBA Ldn, most will be below 60 dBA Ldn.

SAM-EIG HIGHWAY

PLAT NO. 4, 5, 6, 7, 8
PARCEL NO. 1, 2, 3, 4,
102A, 102B, 103
FILE 256, 257 & 267

SHARPSTEAD LANE

DIAMONDBACK DRIVE

SHARPESS ROAD-SOUTH

FIELDS ROAD

MARY HILL LANE

LOT 1

LOT 2

LOT 3

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Traffic Noise Model

Calibration Run

RESULTS: SOUND LEVELS

Phoenix Noise & Vibration
SBH

PROJECT/CONTRACT:
RUN:

BARRIER DESIGN:

ATMOSPHERICS:

68 deg F, 50% RH

N Gaithersburg Investment: Crown Farm

Calibration

INPUT HEIGHTS

30 November 2004
TNM 2.5
Calculated with TNM 2.5

N Gaithersburg Investment: Crown Farm

Average pavement type shall be used unless
a State highway agency substantiates the use
of a different type with approval of FHWA.

Receiver Name	#DUs	Existing L _{Aeq1h} Calculated	No Barrier L _{Aeq1h} Calculated	With Barrier			Noise Reduction Calculated L _{Aeq1h} Sub'l Inc	Type Impact	Goal
				Calculate dB	dB	dB			
A	26	1	0.0	65.1	66	65.1	10	---	65.1
F	30	1	0.0	67.7	66	67.7	10	Snd Lvl	67.7
# DUs Noise Reduction									
			Min	Avg	Max				
All Selected			2	0.0	0.0				0.0
All Impacted			1	0.0	0.0				0.0
All that meet NR Goal			0	0.0	0.0				0.0

INPUT: ROADWAYS

Phoenix Noise & Vibration
SBH

INPUT: ROADWAYS
PROJECT/CONTRACT:
RUN:
Roadway

Average pavement type shall be used unless
a State highway agency substantiates the use
of a different type with the approval of FHWA

30 November 2004
TNM 2.5

N Gaithersburg Investment: Crown Farm**N Gaithersburg Investment: Crown Farm**
Calibration

Roadway Name	Width	Points	Name	No.	Coordinates (pavement)			Flow Control Device	Speed Constraint	Percent Vehicles Affected	% mph	Segment Pvmnt Type	On Struct?
					X	Y	Z						
Sam Eig NB	36.0	point1		1	1,252,468.0	525,985.0	398.00					Average	
		point2		2	1,252,716.0	526,677.0	400.00					Average	
		point3		3	1,252,772.0	526,834.0	402.00					Average	
		point4		4	1,252,907.0	527,205.0	416.00					Average	
		point5		5	1,252,960.0	527,336.0	422.00					Average	
		point6		6	1,253,056.4	527,544.9	432.00					Average	
	36.0	point10		10	1,253,484.0	528,370.0	441.00					Average	
		point11		11	1,253,283.0	528,068.0	441.00					Average	
		point46		46	1,253,143.0	527,837.0	438.00					Average	
		point13		13	1,253,023.8	527,615.2	432.00					Average	
		point147		47	1,253,003.4	527,571.8	432.00	Signal	0.00	100		Average	
	36.0	point14		14	1,252,905.0	527,360.0	422.00					Average	
		point15		15	1,252,851.0	527,226.0	416.00					Average	
		point16		16	1,252,715.0	526,854.0	402.00					Average	
		point17		17	1,252,660.0	526,697.0	400.00					Average	
		point18		18	1,252,411.0	526,005.0	398.00					Average	
Sam Eig NB-2	36.0	point48		48	1,253,084.5	527,602.3	432.00	Signal	0.00	100		Average	
		point7		7	1,253,196.0	527,807.0	438.00					Average	
		point8		8	1,253,333.0	528,036.0	441.00					Average	
		point9		9	1,253,534.0	528,337.0	441.00						

INPUT: TRAFFIC FOR LAeq1h Volumes

**Phoenix Noise & Vibration
SBH**

**INPUT: TRAFFIC FOR LAeq1h Volumes
PROJECT/CONTRACT:**

**RUN:
Roadway**

Name

N Gaithersburg Investment: Crown Farm

Calibration

Roadway	Name	No.	Segment	Motorcycles			Buses			Motorcycles		
				MTrucks			Buses			MTrucks		
				V	S	mph	V	S	mph	V	S	mph
Sam Eig NB	point1	1	1538	45	16	45	16	45	0	0	0	0
	point2	2	1538	45	16	45	16	45	0	0	0	0
	point3	3	1538	45	16	45	16	45	0	0	0	0
	point4	4	1538	45	16	45	16	45	0	0	0	0
	point5	5	1538	45	16	45	16	45	0	0	0	0
	point6	6										
	point10	10	1538	45	16	45	16	45	0	0	0	0
	point11	11	1538	45	16	45	16	45	0	0	0	0
	point46	46	1538	45	16	45	16	45	0	0	0	0
	point13	13										
Sam Eig SB	point47	47	1538	45	16	45	16	45	0	0	0	0
	point14	14	1538	45	16	45	16	45	0	0	0	0
	point15	15	1538	45	16	45	16	45	0	0	0	0
	point16	16	1538	45	16	45	16	45	0	0	0	0
	point17	17	1538	45	16	45	16	45	0	0	0	0
	point18	18										
Sam Eig NB-2	point48	48	1538	45	16	45	16	45	0	0	0	0
	point7	7	1538	45	16	45	16	45	0	0	0	0
	point8	8	1538	45	16	45	16	45	0	0	0	0
	point9	9										

INPUT: RECEIVERS

Phoenix Noise & Vibration
SBH

INPUT: RECEIVERS
PROJECT/CONTRACT:**RUN:****Receiver****Name****N Gaithersburg Investment: Crown Farm****Calibration**

No.	#DUs	Coordinates (ground)			Height above Ground	Input Sound Levels and Criteria			Active in Calc.
		X	Y	Z		Existing	Impact Criteria	NR	
		ft	ft	ft		dBA	dBA		
A	26	1	1,252,930.0	527,714.0	436.00	4.92	0.00	66	10.0
F	30	1	1,252,606.0	526,870.0	409.50	4.92	0.00	66	10.0
								8.0	8.0

N Gaithersburg Investment: Crown Farm

30 November 2004
TNM 2.5

INPUT: BARRIERS

Phoenix Noise & Vibration
PROJECT/CONTRACT:
RUN:

30 November 2004
TNM 2.5

N Gaithersburg Investment: Crown Farm

INPUT: BARRIERS

N Gaithersburg Investment: Crown Farm

Calibration

Barrier	Name	Type	Height Min	Height Max	If Wall \$ per Area \$/sq ft	If Berm \$ per Unit Vol.	Top Width ft	Run:Rise ft/ft	Addtnl \$ per Unit	Length ft	Points Name	No.	Coordinates (bottom) X	Y	Z	Height at Point	Segment Point	Seg Ht	On Incre-	#Up	#Dn	Struct?	Reflec-	tions?
Barrier1		W	0.00	99.99	0.00				0.00	0.00	point1	1	1,252,495.0	526,747.0	413.00	25.00	0.00	0	0	0	0	0	0	0
											point2	2	1,252,539.0	526,731.0	413.00	25.00	0.00	0	0	0	0	0	0	0
											point3	3	1,252,591.0	526,867.0	412.00	25.00	0.00	0	0	0	0	0	0	0
											point4	4	1,252,547.0	526,384.0	412.00	25.00								
Barrier2		W	0.00	99.99	0.00				0.00	0.00	point5	5	1,252,578.0	526,973.0	411.00	25.00	0.00	0	0	0	0	0	0	0
											point6	6	1,252,626.0	526,976.0	411.00	25.00	0.00	0	0	0	0	0	0	0
											point7	7	1,252,629.0	527,073.0	412.00	25.00	0.00	0	0	0	0	0	0	0
											point8	8	1,252,583.0	527,070.0	412.00	25.00								
Barrier3		W	0.00	99.99	0.00				0.00	0.00	point9	9	1,252,592.0	527,102.0	412.00	25.00	0.00	0	0	0	0	0	0	0
											point10	10	1,252,627.0	527,105.0	412.00	25.00	0.00	0	0	0	0	0	0	0
											point11	11	1,252,619.0	527,201.0	413.00	25.00	0.00	0	0	0	0	0	0	0
											point12	12	1,252,581.0	527,199.0	413.00	25.00								
Barrier4		W	0.00	99.99	0.00				0.00	0.00	point13	13	1,252,562.0	527,269.0	416.75	25.00	0.00	0	0	0	0	0	0	0
											point14	14	1,252,588.0	527,298.0	416.75	25.00								
											point15	15	1,252,518.0	527,364.0	422.75	25.00	0.00	0	0	0	0	0	0	0
											point16	16	1,252,482.0	527,335.0	422.75	25.00								
Barrier5		W	0.00	99.99	0.00				0.00	0.00	point17	17	1,252,658.0	527,614.0	426.00	25.00	0.00	0	0	0	0	0	0	0
											point18	18	1,252,745.0	527,617.0	426.00	25.00	0.00	0	0	0	0	0	0	0
											point19	19	1,252,734.0	527,738.0	431.00	25.00	0.00	0	0	0	0	0	0	0
											point20	20	1,252,687.0	527,734.0	431.00	25.00								

INPUT: TERRAIN LINESPhoenix Noise & Vibration
SBH30 November 2004
TNM 2.5**INPUT: TERRAIN LINES**
PROJECT/CONTRACT:

RUN:

Terrain Line**Name****N Gaithersburg Investment: Crown Farm****Calibration**

Terrain Line	Name	No.	Points		
			X ft	Y ft	Z ft
Terrain Line1		1	1,252,906.0	527,761.0	440.00
		2	1,252,997.0	527,701.0	440.00
		3	1,253,027.0	527,739.0	440.00
		4	1,253,112.0	527,878.0	440.00
		5	1,252,912.0	527,866.0	448.00
Terrain Line2		6	1,253,037.0	527,784.0	448.00
		7	1,253,159.0	527,991.0	448.00
		8	1,253,150.0	527,997.0	448.00
		9	1,253,123.0	527,979.0	448.00
		11	1,253,042.0	527,859.0	450.00
Terrain Line3		12	1,253,057.0	527,827.0	450.00
		13	1,253,100.0	527,897.0	450.00
		14	1,253,067.0	527,901.0	450.00

INPUT: GROUND ZONES

Phoenix Noise & Vibration
SBH

INPUT: GROUND ZONES
PROJECT/CONTRACT:

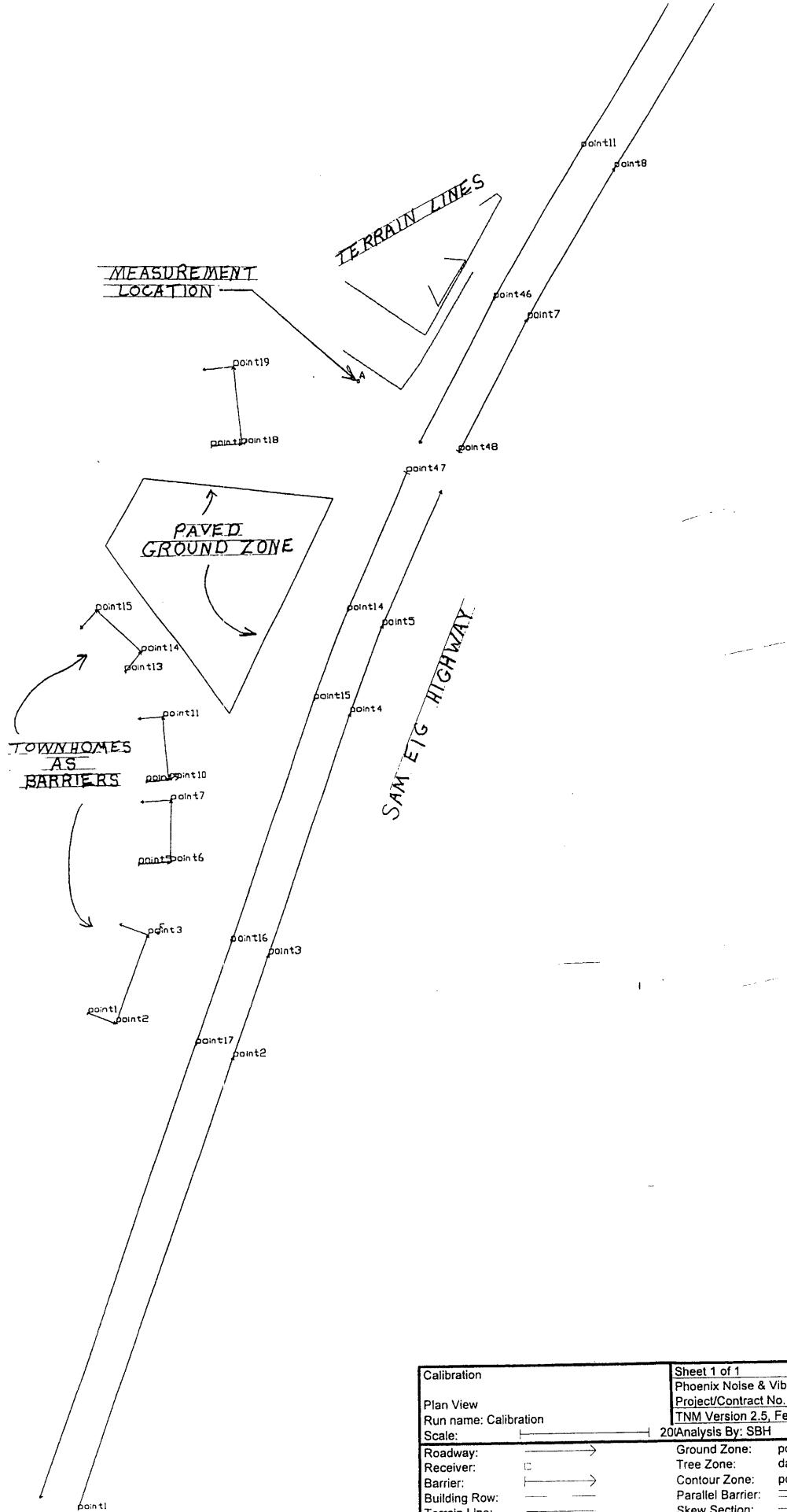
RUN:

N Gaithersburg Investment: Crown Farm**Calibration**

Ground Zone Name	Type	Flow Resistivity cgs rays	Points No.	Coordinates X ft	Y ft
Ground Zone1	Pavement	20000	1	1,252,886.5	527,534.2
			2	1,252,719.5	527,205.6
			3	1,252,534.4	527,463.8
			4	1,252,594.4	527,565.5

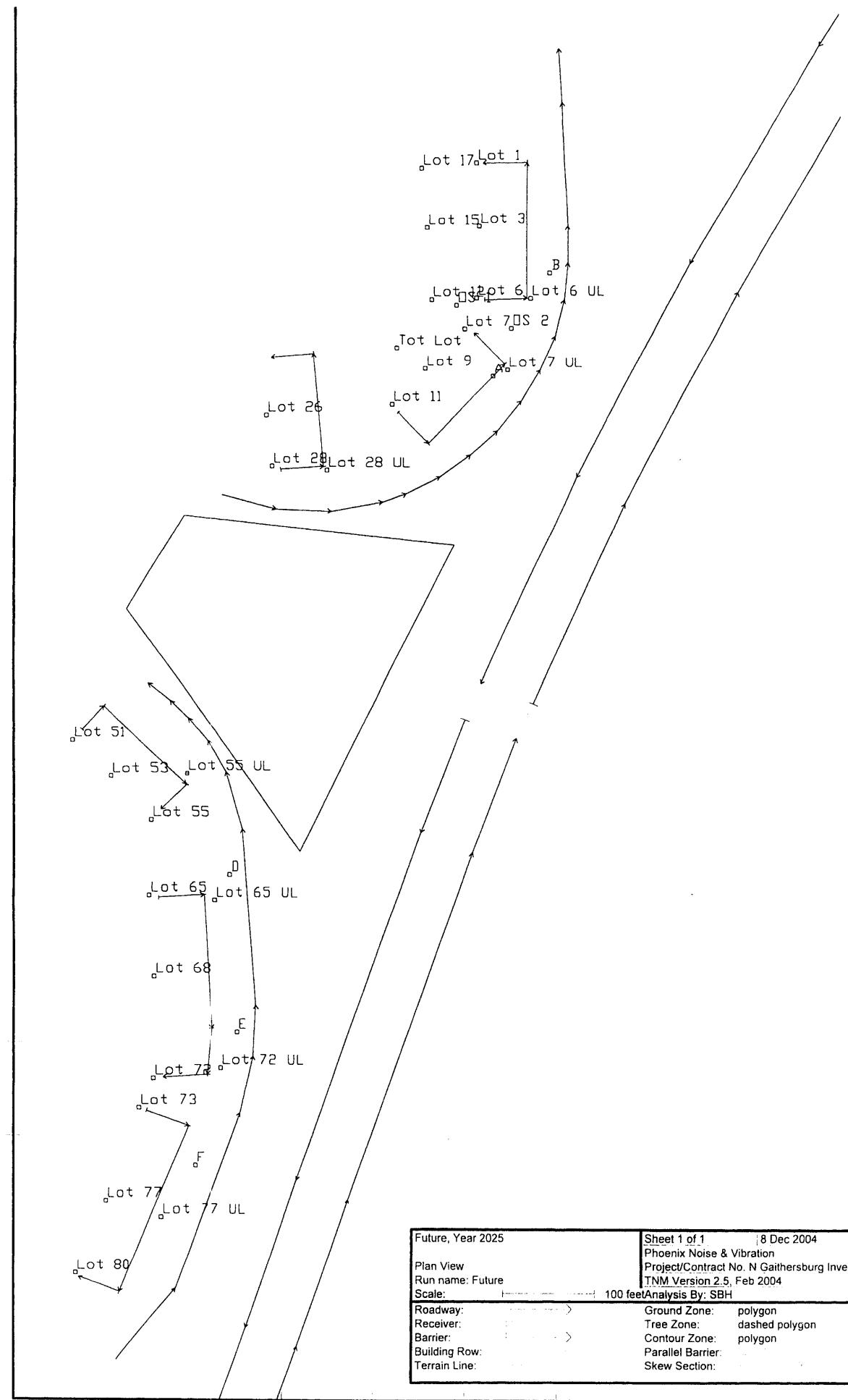
Traffic Noise Model

Future Run



Calibration	Sheet 1 of 1	30 Nov 2004
Plan View	Phoenix Noise & Vibration	
Run name: Calibration	Project/Contract No. N Gaithersburg Investme	
Scale:	TNM Version 2.5, Feb 2004	
Roadway:	→	Ground Zone: polygon
Receiver:	□	Tree Zone: dashed polygon
Barrier:	→	Contour Zone: polygon
Building Row:	— — —	Parallel Barrier: — — —
Terrain Line:	— — —	Skew Section: — →

252400 1252600 1252800 1253000 1253200 1253400 1253600 1253800



1252500 1252600 1252700 1252800 1252900 1253000 1253100 1253200 1253300

RESULTS: SOUND LEVELS

Phoenix Noise & Vibration
SBH

PROJECT/CONTRACT:
RUN:
BARRIER DESIGN:

N Gaithersburg Investment: Crown Farm
Future, Year 2025
INPUT HEIGHTS

ATMOSPHERICS:

Receiver

Name

68 deg F, 50% RH

No.	#DUs	Existing	No Barrier
		L _{Aeq1h}	L _{Aeq1h}
		Calculated	Crit'n

		With Barrier		Without Barrier	
		Increase over existing	Type	Impact	Calculated
		Calculated	Crit'n	L _{Aeq1h}	Calculated
		Sub'l Inc			Goal
1	1	62.4	66	62.4	57.3
2	1	61.8	66	61.8	50.0
3	1	63.6	66	63.6	60.4
4	1	63.9	66	63.9	60.2
5	1	62.4	66	62.4	50.1
6	1	61.3	66	61.3	59.1
7	1	61.4	66	61.4	61.0
8	1	56.4	66	56.4	51.9
9	1	54.4	66	54.4	50.0
12	1	60.8	66	60.8	59.4
13	1	59.4	66	59.4	52.6
14	1	63.3	66	63.3	60.4
15	1	62.8	66	62.8	50.4
16	1	63.6	66	63.6	57.7
17	1	65.4	66	65.4	61.7
18	1	63.5	66	63.5	50.2
19	1	61.2	66	61.2	54.5
20	1	57.4	66	57.4	53.3
21	1	59.0	66	59.0	53.3
22	1	60.9	66	60.9	54.4
23	1	62.5	66	62.5	58.9
24	1	66.9	66	66.9	66.8
Total		61.2	66	61.2	56.4
OS 1		0.0	0.0	0.0	0.1
OS 2		0.0	0.0	0.0	0.8
Lot 12		0.0	0.0	0.0	-3.2

Average pavement type shall be used unless
a State highway agency substantiates the use
of a different type with approval of FHWA.

8 December 2004

TNM 2.5
Calculated with TNM 2.5

RESULTS: SOUND LEVELS

	N Gaithersburg Investment: Crown Farm		
	10 Snd Lvl	10 Snd Lvl	10 Snd Lvl
A	68.0	68.0	68.0
B	68.1	68.1	68.1
D	68.1	68.1	68.1
E	64.8	66	64.8
F	68.5	66	68.5
Lot 6 UL	68.7	66	68.7
Lot 7 UL	68.2	66	68.2
Lot 28 UL	68.7	66	68.7
Lot 65 UL	65.4	66	65.4
Lot 72 UL	66	66	66
Lot 55 UL	66.9	66	66.9
Lot 77 UL	69.0	66	69.0
	65.2	66	65.2
	68.5	66	68.5
	68.5	66	68.5

Dwelling Units

DUs Noise Reduction

	Min dB	Avg dB	Max dB
All Selected	35	0.0	13.3
All Impacted	10	0.0	0.2
All that meet NR Goal	4	11.8	12.4

INPUT: ROADWAYS

Phoenix Noise & Vibration
SBH

INPUT: ROADWAYS
PROJECT/CONTRACT:

RUN:

Roadway

Name

**N Gaithersburg Investment: Crown Farm
Future, Year 2025**

8 December 2004
TNM 2.5

N Gaithersburg+Investment: Crown Farm

Average pavement type shall be used unless
a State highway agency substantiates the use
of a different type with the approval of FHWA

Roadway Name	Width	Points	No.	Coordinates (pavement)			Flow Control			Segment Pymt Type	Percent Vehicles Affected	On Struct?
				X	Y	Z	Control Device	Speed Constraint				
	ft		ft	ft	ft	ft	mph	%				
Sam Eig NB	36.0	point1	1	1,252,468.0	525,985.0	398.00				Average Pavmt Type	Average %	Average On Struct?
		point2	2	1,252,716.0	526,677.0	400.00						
		point3	3	1,252,772.0	526,834.0	402.00						
		point4	4	1,252,907.0	527,205.0	416.00						
		point5	5	1,252,955.1	527,326.3	422.00						
Sam Eig SB	36.0	point10	10	1,253,484.0	528,370.0	441.00				Average Pavmt Type	Average %	Average On Struct?
		point11	11	1,253,283.0	528,068.0	441.00						
		point12	12	1,253,143.0	527,837.0	438.00						
		point13	13	1,253,020.0	527,607.0	432.00						
		point14	14	1,252,915.9	527,384.3	422.00						
Norwich Lane	24.0	point19	19	1,252,634.0	527,586.9	424.00				Average Pavmt Type	Average %	Average On Struct?
		point20	20	1,252,693.1	527,572.3	423.00						
		point21	21	1,252,753.5	527,570.3	424.00						
		point22	22	1,252,809.2	527,580.1	424.00						
		point23	23	1,252,835.0	527,589.4	424.00						
		point24	24	1,252,871.8	527,607.2	426.00				Average Pavmt Type	Average %	Average On Struct?
		point25	25	1,252,905.4	527,630.0	428.00						
		point26	26	1,252,934.6	527,656.3	430.00						
		point27	27	1,252,960.6	527,687.9	432.00						
		point28	28	1,252,981.2	527,721.9	434.00						
		point29	29	1,252,996.9	527,758.9	436.00				Average Pavmt Type	Average %	Average On Struct?
		point30	30	1,253,006.8	527,797.5	438.00						
		point31	31	1,253,011.1	527,836.6	440.00						
		point32	32	1,253,010.8	527,876.3	442.00						
		point33	33	1,253,004.0	528,007.0	448.00						

INPUT: ROADWAYS**N Gaithersburg Investment: Crown Farm**

		point34	34	1,253,000.0	528,063.0	448.00	Average
Sharpstead Lane	24.0	point35	35	1,252,522.0	526,662.4	410.00	Average
		point36	36	1,252,585.5	526,739.2	410.00	Average
		point37	37	1,252,653.9	526,926.5	408.00	Average
		point38	38	1,252,668.6	526,986.3	408.00	Average
		point39	39	1,252,671.8	527,041.2	408.00	Average
		point40	40	1,252,657.9	527,229.4	410.00	Average
		point41	41	1,252,639.8	527,289.1	412.00	Average
		point42	42	1,252,619.9	527,323.0	414.00	Average
		point43	43	1,252,599.5	527,346.3	416.00	Average
		point44	44	1,252,580.2	527,365.5	418.00	Average
		point45	45	1,252,557.5	527,383.6	420.00	Average
		point46	46	1,252,898.9	527,346.7	422.00	Signal
Sam Eig SB-2	36.0	point15	15	1,252,851.0	527,226.0	416.00	Average
		point16	16	1,252,715.0	526,854.0	402.00	Average
		point17	17	1,252,660.0	526,697.0	400.00	Average
		point18	18	1,252,411.0	526,005.0	398.00	Average
Sam Eig NB-2	36.0	point47	47	1,252,973.4	527,363.9	422.00	Signal
		point6	6	1,253,073.0	527,580.0	432.00	Average
		point7	7	1,253,196.0	527,807.0	438.00	Average
		point8	8	1,253,333.0	528,036.0	441.00	Average
		point9	9	1,253,534.0	528,337.0	441.00	Average

INPUT: TRAFFIC FOR LAeq1h Volumes

Phoenix Noise & Vibration
SBH

8 December 2004
TNM 2.5

N Gaithersburg Investment: Crown Farm**INPUT: TRAFFIC FOR LAeq1h Volumes****PROJECT/CONTRACT:****RUN:****N Gaithersburg Investment: Crown Farm****Future, Year 2025**

Roadway Name	Points	Name	No.	Segment	Motorcycles			Buses			Motorcycles			
					Autos		MTrucks		HTrucks		V		V	
					V	S	V	S	V	S	V	S	V	S
					veh/hr	mph	veh/hr	mph	veh/hr	mph	veh/hr	mph	veh/hr	mph
Sam Eig NB	point1		1	2023	45	21	45	21	45	21	45	0	0	0
	point2		2	2023	45	21	45	21	45	21	45	0	0	0
	point3		3	2023	45	21	45	21	45	21	45	0	0	0
	point4		4	2023	45	21	45	21	45	21	45	0	0	0
	point5		5											
	point10		10	2023	45	21	45	21	45	21	45	0	0	0
	point11		11	2023	45	21	45	21	45	21	45	0	0	0
	point12		12	2023	45	21	45	21	45	21	45	0	0	0
	point13		13	2023	45	21	45	21	45	21	45	0	0	0
	point14		14											
	point19		19		0	0	0	0	0	0	0	0	0	0
	point20		20		0	0	0	0	0	0	0	0	0	0
	point21		21		0	0	0	0	0	0	0	0	0	0
	point22		22		0	0	0	0	0	0	0	0	0	0
	point23		23		0	0	0	0	0	0	0	0	0	0
	point24		24		0	0	0	0	0	0	0	0	0	0
	point25		25		0	0	0	0	0	0	0	0	0	0
	point26		26		0	0	0	0	0	0	0	0	0	0
	point27		27		0	0	0	0	0	0	0	0	0	0
	point28		28		0	0	0	0	0	0	0	0	0	0
	point29		29		0	0	0	0	0	0	0	0	0	0
	point30		30		0	0	0	0	0	0	0	0	0	0

INPUT: TRAFFIC FOR LAeq1h Volumes

N Gaithersburg Investment: Crown Farm									
point31	31	0	0	0	0	0	0	0	0
point32	32	0	0	0	0	0	0	0	0
point33	33	0	0	0	0	0	0	0	0
point34	34	0	0	0	0	0	0	0	0
point35	35	0	0	0	0	0	0	0	0
point36	36	0	0	0	0	0	0	0	0
point37	37	0	0	0	0	0	0	0	0
point38	38	0	0	0	0	0	0	0	0
point39	39	0	0	0	0	0	0	0	0
point40	40	0	0	0	0	0	0	0	0
point41	41	0	0	0	0	0	0	0	0
point42	42	0	0	0	0	0	0	0	0
point43	43	0	0	0	0	0	0	0	0
point44	44	0	0	0	0	0	0	0	0
point45	45	0	0	0	0	0	0	0	0
point46	46	2023	45	21	45	21	45	0	0
point15	15	2023	45	21	45	21	45	0	0
point16	16	2023	45	21	45	21	45	0	0
point17	17	2023	45	21	45	21	45	0	0
point18	18							0	0
point47	47	2023	45	21	45	21	45	0	0
point6	6	2023	45	21	45	21	45	0	0
point7	7	2023	45	21	45	21	45	0	0
point8	8	2023	45	21	45	21	45	0	0
point9	9							0	0

INPUT: RECEIVERS

Phoenix Noise & Vibration
SBH

INPUT: RECEIVERS
PROJECT/CONTRACT:

RUN:

Receiver
Name

N Gaithersburg Investment: Crown Farm
Future, Year 2025

8 December 2004
TNM 2.5

N Gaithersburg Investment: Crown Farm

No.	#DUs	Coordinates (ground)			Height above Ground	Input Sound Levels and Criteria			Active in Calc.		
		X	Y	Z		Existing LAeq1h	Impact Criteria LAeq1h	NR Sub'l	Goal	dBA	dB
1	1	1,252,478.2	526,755.6	412.50	4.92	0.00	66	10.0	8.0		
Lot 80	1	1,252,510.6	526,831.7	411.00	4.92	0.00	66	10.0	8.0		
Lot 77	1	1,252,547.0	526,931.5	411.50	4.92	0.00	66	10.0	8.0		
Lot 73	1	1,252,562.5	526,963.3	410.50	4.92	0.00	66	10.0	8.0		
Lot 72	1	1,252,562.8	527,071.0	410.50	4.92	0.00	66	10.0	8.0		
Lot 68	1	1,252,558.0	527,158.4	410.50	4.92	0.00	66	10.0	8.0		
Lot 65	1	1,252,560.2	527,238.2	411.00	4.92	0.00	66	10.0	8.0		
Lot 55	1	1,252,517.5	527,284.4	411.50	4.92	0.00	66	10.0	8.0		
Lot 53	1	1,252,475.0	527,322.8	412.00	4.92	0.00	66	10.0	8.0		
Lot 51	1	1,252,688.2	527,617.1	425.00	4.92	0.00	66	10.0	8.0		
Lot 28	1	1,252,682.9	527,671.7	427.00	4.92	0.00	66	10.0	8.0		
Lot 26	1	1,252,855.6	527,722.3	430.00	4.92	0.00	66	10.0	8.0		
Lot 11	1	1,252,898.6	527,765.1	433.00	4.92	0.00	66	10.0	8.0		
Lot 9	1	1,252,912.2	527,798.8	440.50	4.92	0.00	66	10.0	8.0		
Lot 7	1	1,252,914.9	527,875.1	444.50	4.92	0.00	66	10.0	8.0		
Lot 6	1	1,252,911.9	527,941.6	445.80	4.92	0.00	66	10.0	8.0		
Lot 3	1	1,252,857.4	527,872.4	437.00	4.92	0.00	66	10.0	8.0		
Lot 1	1	1,252,824.2	527,744.8	429.00	4.92	0.00	66	10.0	8.0		
Lot 17	1	1,252,890.2	527,791.0	434.00	4.92	0.00	66	10.0	8.0		
Lot 15	1	1,252,949.6	527,766.7	436.00	4.92	0.00	66	10.0	8.0		
Tot Lot											
OS 1											
OS 2											

INPUT: RECEIVERS

N Gaithersburg Investment: Crown Farm										
INPUT: RECEIVERS										
Lot 12		25	1	1,252,863.0	527,797.0	433.00	4.92	0.00	66	10.0
A		26	1	1,252,930.0	527,714.0	436.00	4.92	0.00	66	10.0
B		27	1	1,252,991.0	527,826.0	448.00	4.92	0.00	66	10.0
D		28	1	1,252,643.0	527,180.0	410.00	4.92	0.00	66	10.0
E		29	1	1,252,651.0	527,012.0	408.50	4.92	0.00	66	10.0
F		30	1	1,252,606.0	526,870.0	409.50	4.92	0.00	66	10.0
Lot 6 UL		32	1	1,252,970.8	527,798.7	440.50	25.00	0.00	66	10.0
Lot 7 UL		33	1	1,252,946.2	527,721.9	433.00	25.00	0.00	66	10.0
Lot 28 UL		34	1	1,252,748.9	527,612.9	425.00	25.00	0.00	66	10.0
Lot 65 UL		35	1	1,252,627.0	527,153.4	411.00	25.00	0.00	66	10.0
Lot 72 UL		36	1	1,252,633.2	526,974.0	410.50	25.00	0.00	66	10.0
Lot 55 UL		37	1	1,252,597.8	527,287.6	416.00	25.00	0.00	66	10.0
Lot 77 UL		38	1	1,252,569.9	526,814.8	411.00	25.00	0.00	66	10.0

N Gaithersburg Investment: Crown Farm

N Gaithersburg Investment: Crown Farm

INPUT: BARRIERS

Phoenix Noise & Vibration
PROJECT/CONTRACT:
RUN:

8 December 2004
TBM 2.5

N Gaithersburg Investment: Crown Farm

N Gaithersburg Investment: Crown Farm
Future, Year 2025

Barrier Name	Type	Height	If Wall		If Berm		Addtnl \$ per Unit Length	Name	Coordinates (bottom)			Segment ft	Height at Point ft	Important Seg Ht Perturb On Incre-#Up Struct? Reflec-tions?
			Min	Max	\$ per Unit Area	\$ per cu yd			X	Y	Z			
Barrier5	W	0.00	99.99	0.00			0.00	point17	17	1,252.698.0	527.614.0	426.00	25.00	1.00
								point18	18	1,252.745.0	527.617.0	426.00	25.00	1.00
								point19	19	1,252.734.0	527.738.0	431.00	25.00	1.00
								point20	20	1,252.687.0	527.734.0	431.00	25.00	
Barrier6	W	0.00	99.99	0.00			0.00	point21	21	1,252.826.0	527.675.0	429.00	25.00	1.00
								point22	22	1,252.859.0	527.642.0	429.00	25.00	1.00
								point23	23	1,252.943.0	527.728.0	434.00	25.00	1.00
								point24	24	1,252.909.0	527.762.0	434.00	25.00	
Barrier7	W	0.00	99.99	0.00			0.00	point25	25	1,252.921.0	527.797.0	441.00	25.00	1.00
								point26	26	1,252.987.0	527.799.0	441.00	25.00	1.00
								point27	27	1,252.987.0	527.943.0	446.50	25.00	1.00
								point28	28	1,252.919.0	527.942.0	446.50	25.00	
								point29	29	1,252.955.0	526.929.0	412.00	25.00	1.00
								point30	30	1,252.999.0	526.913.0	412.00	25.00	
Barrier8	W	0.00	99.99	0.00			0.00	point31	31	1,252.926.0	526.735.0	413.00	25.00	1.00
								point32	32	1,252.981.0	526.751.0	413.00	25.00	
								point33	33	1,252.968.0	527.156.0	412.50	25.00	1.00
Barrier9	W	0.00	99.99	0.00			0.00	point34	34	1,252.916.0	527.159.0	412.50	25.00	1.00
								point35	35	1,252.624.0	527.016.0	411.50	25.00	1.00
								point36	36	1,252.619.0	526.967.0	411.00	25.00	1.00
								point37	37	1,252.572.0	526.964.0	411.00	25.00	
Barrier10	W	0.00	99.99	0.00			0.00	point38	38	1,252.485.0	527.334.0	422.75	25.00	1.00
								point39	39	1,252.509.0	527.360.0	422.75	25.00	1.00
								point40	40	1,252.597.0	527.276.0	418.00	25.00	1.00
								point41	41	1,252.570.0	527.249.0	418.00	25.00	

INPUT: GROUND ZONES

Phoenix Noise & Vibration
SBH

INPUT: GROUND ZONES
PROJECT/CONTRACT:

RUN:

Ground Zone

Name

**N Gaithersburg Investment: Crown Farm
Future, Year 2025**

8 December 2004
TNM 2.5

Ground Zone	Name	Type	Flow Resistivity cgs rayls	Points	
				No.	Coordinates X ft
Ground Zone1		Pavement	200000	1	1,252,886.5 527,534.2
				2	1,252,719.5 527,205.6
				3	1,252,534.4 527,463.8
				4	1,252,594.4 527,565.5

INPUT: CONTOUR ZONES

Phoenix Noise & Vibration
SBH

INPUT: CONTOUR ZONES**PROJECT/CONTRACT:****RUN:****Contour Zone Name****N Gaithersburg Investment: Crown Farm
Future, Year 2025****8 December 2004
TNM 2.5****N Gaithersburg Investment: Crown Farm**

Contour Zone	Name	Grid Height	Grid Spacing ft	Minimum Grid Spacing ft	Contour Tolerance dB	Points	
						No.	Coordinates X ft
Contour Zone2			25.00	200.00	1	1	1,253,268.1 ft
						2	1,253,072.4 ft
						3	1,252,927.2 ft
						4	1,252,454.5 ft
						5	1,252,874.4 ft
Contour Zone3			25.00	200.00	1	6	1,252,508.0 ft
						7	1,252,775.9 ft
						8	1,252,560.5 ft
						9	1,252,303.0 ft
							526,570.4 ft
							526,747.8 ft